A report prepared for

HDR Engineering, Inc. 2525 C St., Suite 305 Anchorage, Ak 99503

GEOTECHNICAL INVESTIGATION Airport Material Sites Ñoorvik, Alaska

by

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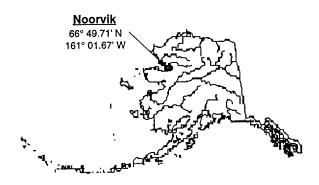
INTRODUCTION

The State of Alaska Department of Transportation and Public Facilities (DOT&PF) is planning to build a new airport complex near the community of Noorvik. Bank erosion along the Kobuk River is threatening the existing facility. Sources of unclassified and select material are needed for this project.

Historically, fine grained material has been obtained from an island sand bar upstream from Noorvik, from a point bar right in front of the community and from a nearby lake bottom. The lake bottom material was of poor quality and the other sites are either exhausted or no longer available for use.

A small DOT&PF materials site is present approximately six miles east of the community of Noorvik, near the western base of Hotham Peak. Access from Noorvik is by winter trail only. Material from this site was used successfully in construction of the airport extension. Because this pit is located so far from town and because it is presently accessible only in late winter, via an ice/snow road, it has not been utilized since the work on the airstrip was completed.

Suitable material could be imported but this would be expensive and may require several seasons to accomplish. Local source options include the river bar deposits near the summer dump, just downstream from the community (DOT&PF Site 3) and the terrace deposits adjacent to the existing Hotham Peak pit (DOT&PF Site D). The present investigation was undertaken in an effort to expand our knowledge of the soil characteristics at these two sites and to determine if sufficient usable embankment material is locally available to construct the proposed new airport. The project areas are located on land controlled by the City of Noorvik and the NANA Native Corporation.



REGIONAL SETTING

The Kobuk River Delta forms part of the Kobuk-Selawik Lowland which consists primarily of the broad flood plains and deltas of the Kobuk and Selawik Rivers and the intervening lake dotted lowlands. Most of the lowland areas are underlain by moraine deposits which are often mantled by thick stream and lake deposits. Away from the active river channels, thick organic surface deposits are common. Permafrost is present throughout the area but is discontinuous near large bodies of water. Drainage is often poor due to the low topographic gradient. Most streams on the delta are sluggish and meandering and have numerous side sloughs.

East of the delta, undifferentiated glacial and glacio-fluvial deposits have resulted in a gently rolling terrain with rounded knolls. Relief is on the order of 50 to 100 feet. This subdued glacial terrain is generally overlain by thick sequences of wind blown silt and organic material. Further to the east, Hotham Peak (which rises to over 1600 feet) is composed of Cretaceous sedimentary rock.

The climate in the Noorvik area is influenced somewhat by the maritime storms from the west but overall precipitation is very low, and winter conditions are generally cold and windy. The Environmental Atlas of Alaska, 1978, shows a mean average temperature of 21.5° F. Annual precipitation is only about 8 inches but up to four feet of annual snowfall is normal. Snow accumulation on the ground, however, is much less due to persistent winter winds and snow drifting can be significant.

Vegetation in the area is comprised primarily of wet tundra plants, sedges and dwarf shrubs. Black spruce grow on the delta and birch, willow and alder groves are present along major drainage areas.

EXPLORATION PLAN

The present geotechnical investigation has been undertaken in an effort to expand our knowledge of the soil characteristics on the river bar downstream from the community and on the terrace in the vicinity of the existing Hotham Peak pit. Our goal was to determine if earthen material suitable for access road and airport runway construction is locally available. Material is needed to construct the proposed new airport as well as the six-mile all weather road that will be needed to make operation of the Hotham Peak site feasible as a year round source of material.

In preparation for the field work we reviewed existing DOT&PF information, studied existing airphoto imagery, and interviewed several local residents to gain an overall perspective of the project area.

A 1973 field study by the State of Alaska Division of Aviation (now DOT&PF) was the first documented material source study in the Hotham Peak area. Fine sandy gravel and gravelly sand were reported to be present just west of the main drainage way that runs at the base of Hotham Peak. Three test holes, drilled to 35 feet as part of that project, encountered 1.5 to 3.75 feet of silty overburden over frost bonded granular material. The locations of the holes are shown on Plate HP-1. On the basis of that study it was estimated that 100,000 cubic yards of material suitable for topping was available.

A report describing the results of a 1976 DOT&PF materials source study in the Noorvik area indicated that material near the river (DOT&PF Site 3) contained a higher percentage of sand than that at the airport or in the nearby lake bottom borrow source. The 1976 report also referred to the Hotham Peak source but mentioned no new field work in that area. That report did note that at least one sample collected in 1973 had 13% fines which did not meet the specification for topping material.

From an airplane in May and then on foot in August, 1996, Matt Desalernos (DOT&PF Western District Design Engineer) conducted a field reconnaissance of the Hotham Peak area and concluded that the potential was high for expanding the existing pit and, if necessary, opening new pits on similar topographic features.

Two drilling plans were laid out: Phase 1 would extend our knowledge of subsurface conditions at the existing DOT&PF Site 3 for a mile or more downstream from Noorvik, and Phase 2 would evaluate the granular material and overburden silt at the Hotham Peak Site (DOT&PF Site D).

Phase 1 was limited to exploring for fine grained fill material on the low river terrace immediately downstream from town. The Phase 1 field work was scheduled for late November, 1996, using a small back-pack drill, as outlined in the exploration plan submitted to HDR on October 6, 1996.

The observations of Mr. Desalernos, supported by the stereo evaluation of 1993 color aerial photos (1″=833′), led to the formulation of the Phase 2 study plan. The second phase focused primarily on the coarser granular at the Hotham Peak site. In addition, an evaluation of the overburden silt at that site was made to determine its suitability for use as unclassified fill. Phase 2 work was originally scheduled for February or March, 1997 but because of local equipment difficulties it was rescheduled for April. The work was conducted in general accordance with the revised exploration plan submitted to HDR on March 11, 1997. In Phase 2, a standard geotechnical drill capable of drilling to 40 feet in frozen granular material and equipped for penetration testing was specified. Site preparation and drill transport was to be accomplished with a tracked dozer.

The vicinity map (Plate 1) at the end of this report shows the relationship of each of the study areas to the community of Noorvik and to the proposed airport facility.

During the planning and execution of each phase we coordinated activities with Mr. Duane Hippe, P.E., of HDR. Ms. Susan Walker of the HDR Anchorage office coordinated our activities with DOT&PF and provided permitting and logistical support. Mr. Glen Skin, Noorvik City Administrator, was very helpful in arranging for local hire and housing support.

Mr. Walter T. Phillips, PG was in charge of the field activities during the Phase 1 work and Mr. Ethan Birkholtz, DOT&PF Northern Region, was on site during much of that investigation. Mr. Thomas Pungalik acted as local guide, snow machine driver, and driller's helper. In Phase 2, Mr. Thomas Culkin supervised the field work, accompanied by Mr. Matt Desalernos from DOT&PF.

Mr. Elmer Pungalik acted as local guide & snow machine driver and Mr. Fred Jack directed the K. I. C. activities related to drill rig transport during the April, 1997 program.

NAZURUK CHANNEL SAND BAR SITE

Location and Access

The Sand Bar Site is a southerly extension of an existing DOT&PF material source (Site 3) which is located on the eastern edge of the Nazuruk Channel, just south of Noorvik. The intent was to explore and hopefully extend this sand source for a mile or so downstream. Early in the investigation, however, it became apparent that soil conditions to the south were less suitable then anticipated so the focus of the investigation was altered (in consultation with Mr. Birkholtz) to better delineate the more suitable soils north of the existing Summer Dump.

The existing road to the dump provides access to this borrow area from downtown Noorvik. Winter trails would have to be improved to allow summer haulage to the proposed airport site.

Historic Use

Embankment material for the present airport was taken in part from the low river terrace just north of town. The more recent construction of the cross-wind runway also utilized material from a similar terrace immediately downstream from town (DOT&PF Site 3).

Field Exploration

Field work began on November 12, 1996. Over a period of three days, new subsurface information was obtained at 16 locations in the vicinity of the proposed material site. Depth of exploration ranged from 3 to 18 feet, depending on subsurface conditions. Plate NC-1 shows the location of each boring.

Terrain conditions and time constraints prevented the use of heavy, mechanized equipment over much of the area, so subsurface exploration was limited to what could be accomplished with a hand-held back-pack drill. A 2.5-horsepower powerhead drill, owned and operated by Inanda Placer of Fairbanks was used for drilling and sampling. Sampling was conducted by grabbing samples from the auger bit and by coring using a 2-inch O.D. split barrel with a

carbide tipped bit. A 2-inch auger with a 2.5-inch cutting head was used. Depth of investigation was sometimes limited by drill capabilities.

At key locations, geographic coordinates (latitude and longitude) were obtained with a hand held GPS unit. Each exploration site was located either by GPS, by reference to a GPS located hole or by airphoto interpretation.

Soil type, frost and ground water conditions were noted as each hole was drilled and representative soil samples were collected. Descriptive logs of each boring presented on Plates NC-2 through NC-10. The soils and ice have been classified in accordance with the Unified Soil Classification System presented on Plate NC-11. The boring logs also show the technique used for the collection of each sample.

Laboratory Testing

At the drill site the samples were sealed in plastic bags for shipment to Anchorage for further study. The samples were visually examined in the laboratory to verify the field description and representative samples were selected to test for grain size distribution and natural moisture content. Certain samples were also tested for organic content and representative samples of the silty sand and sandy silt were then combined and tested to determine optimum moisture and density values using a Harvard miniature compaction test.

The results of the natural moisture content tests are graphically shown on the boring logs and are tabulated on the Summary of Samples (Plates NC-12 and NC-13) along with the other test values. The particle size data have been analyzed for size breaks, uniformity and curvature; results of this analysis are presented on Plate NC-14. The optimum moisture and compaction test results are shown on Plate NC-15.

Drainage and Hydrology

Surface drainage on this river terrace is fair to good depending on the underlying thermal condition. Vegetation is sparse on the active bar immediate adjacent to the river and the subsoil is drained, even where frozen. Older portions of the terrace generally support an organic mat that is sometimes a foot or more in thickness. Subsurface moisture contents tend to be higher and a thick willow growth is present.

Immediately west of the summer dump and to the south, an old slough channel is present. This provides limited surface drainage and the material in the channel is saturated beneath a depth of about two feet.

Thermal Conditions

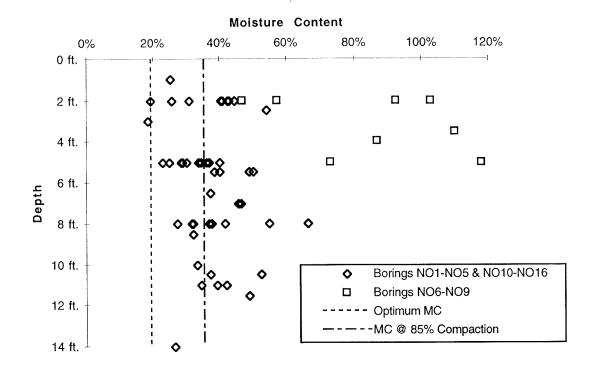
The material in the active bar and north of the summer dump is frozen silt and sandy silt. The material in the old slough channel is saturated unfrozen silt. South of the summer dump and inland from the active bar and intervening slough channel the underlying silt is, in general, frozen but it is ice rich and often contains a significant organic content.

Cobbles and Boulders

Sedimentary accumulation on this deltaic terrace has been limited to fine grained material. No cobbles or boulders were noted during the field investigation.

Quality

The silty sands reported in DOT&PF Site 3 appear to be confined primarily to the recently active river bar that extends to the south along the river edge for half a mile or so. As much as 12 feet of interbedded silty sand and sandy silt is present in this two to three hundred foot wide bar. The percentage of sand lessens somewhat to the south and east. The natural moisture content of the frozen sandy material ranges up to 40%. Interbedded silty layers have moisture contents of 60% or higher. Optimum moisture is on the order of 20% with 35% moisture corresponding to the upper bound for obtaining a relative compaction of 85%. The maximum density obtained in the lab at optimum moisture was 101 pcf. The chart on the following page shows the relationship between natural moisture content and depth for the sand and silt units in the area. Selective mining of the sandy units will be necessary to efficiently utilize this material site.



Inland from the active bar and south of the summer dump, silt and organic silt are the predominant soil types, and segregated ice and moisture contents are generally higher.

Quantity

The usable material in the area north and west of the dump is generally six to twelve feet thick. Overburden ranges from nil next to the river to about one foot at the eastern edge of the lower terrace. Overburden could be stored along the eastern edge of the terrace and it might be used to close out and restore the summer dump.

The sandy silt in the bar next to the river can be mined, but the suitability of material to the east may be limited due to the increased silt content and the presence of wet, unfrozen zones. Wet, unfrozen zones near the drainage swale could make winter excavation more difficult.

If the material from this area is placed in a frozen condition, considerable settlement and temporary loss of strength will occur when it is first thawed.

HOTHAM PEAK TERRACE SITE

Location and Access

A small DOT&PF materials site is located approximately six miles east of the community of Noorvik, near the western base of Hotham Peak. Present access is limited to a six-mile winter trail across the tundra. Due to poor surface drainage in some areas between Noorvik and Hotham Peak, it was necessary to schedule the drill program for late winter to take advantage of the frozen ground and snow cover for heavy equipment transport.

Historic Use

Material from this site was used successfully in construction of the airport extension. Because this pit is located so far from town and because it is presently accessible only in late winter via an ice/snow road, it has not been utilized since the work on the airstrip was completed.

Field Exploration

On April 2, 1997, the drill rig arrived in Noorvik by air, and the drill was moved on site. Following a one day weather delay, subsurface exploration began on April 4th.

The drill, a CME-45, was mounted on a sled. A second smaller sled was used to carry augers and other drill equipment. The drill equipment was supplied and operated by Discovery Drilling of Anchorage. Logistical support was provided by K.I.C. from their base of operations at Kotzebue. Two snow machines and a Caterpillar D-3 were used for transport. The drill rig was moved from site to site by the D-3 and a snow machine pulled the tool sled. The crews were housed in Noorvik and commuted to the project area by snow machine.

Between April 4 and 7, 1997, eleven borings in the vicinity of the proposed material site were drilled and sampled. In addition, the cut bank of the existing pit was examined. Depth of exploration ranged from 6 to 39 feet, depending on subsurface conditions and drill capabilities.

At key locations, geographic coordinates (latitude and longitude) were obtained with a hand held GPS unit. Each exploration site was located either by

GPS, by reference to a GPS located hole or by aerial photo interpretation to geographical features on the ground. The location of each exploration site is shown on Plate HP-1.

Holes were drilled with either 3.5-inch O.D. solid stem or 8-inch O.D. hollow stem augers. As each hole was drilled, the soil type and frost conditions were noted and representative soil samples were collected. Samples of the soil were obtained by driving a 2.5-inch I.D. split barrel sampler and by grab sampling off the augers. The CME-45 was equipped with a cathead and a manual drop hammer. The hammer weighed 340 pounds and had a drop of 30 inches, and blow counts for each 6-inch increment of the drive were recorded. Sampling was generally attempted at five-foot intervals. Drill behavior was also used to estimate changes in strata; cobbles in the strata were logged on the basis of hard and irregular drilling behavior. The samples were sealed in plastic bags for shipment to the laboratory for further evaluation and testing.

A graphic log of each boring drilled in 1997 is presented on Plates HP-2 through HP-12. Re-drafted logs of the three holes drilled by DOT&PF in 1973 (TH-2, 3 & 4) are shown on Plates HP-13 through HP-15. A copy of the original DOT&PF logs is shown on Plate HP-20 and is followed by a copy of the 1973 laboratory data. The soils and ice have been classified on the 1997 logs in accordance with the Unified Soil Classification System presented on Plate HP-16. The boring logs show the type of sampler used and the equivalent SPT-N value of the field blow counts. The field blow counts have been adjusted for the hammer weight and sampler annulus. Each hole was backfilled upon completion. The site was cleaned and all refuse, from lunch bags to broken equipment parts, was carried back to Noorvik for disposal.

Drilling at the Hotham Peak site was quite difficult and midway through the drilling project, a crack in the tower base developed which made it unsafe to apply the downhole pressure needed to penetrate the granular material. As a result of this strength limitation, a number of test holes were terminated at the base of the silt cover and proof of quantity was not obtained for the granular deposit. Sampling capabilities were also limited.

Laboratory Testing

Laboratory testing was conducted in Anchorage. Primary testing included natural moisture content determinations for most samples, visual classification and classification tests (particle size analysis) and specific gravity tests. Degradation, L. A.. Abrasion and modified Proctor (D1557) compaction tests were also run on certain representative samples. Preliminary moisture testing and visual classification was performed by DM&A; other laboratory testing was done by R&M Consultants. The results of the natural moisture content tests are graphically shown on the boring logs and are tabulated on the Summary of Samples (Plate HP-17) along with the other test values. The particle size data have been analyzed for size breaks, uniformity and curvature; results of this analysis are presented on Plates HP-18 and HP-19.

Degradation, L. A.. Abrasion and modified Proctor (D1557) compaction tests were performed on the bulk samples obtained from borings ORV-10 and ORV-11. The degradation value is 38 which is less than the required minimum value of 45 according to the Alaska DOT&PF Standard Specifications for base coarse aggregate, Section 703-2.03. The L. A.. Abrasion value is 28% loss, which is less than the maximum value of 50% according to the same specification. The modified Proctor (D1557) compaction test showed a maximum dry density of 136 pcf at an optimum moisture content of 7.5%. All of the above test data are presented as attachments at the end of this report.

Drainage & Hydrology

Surface drainage on this high terrace is well defined. Subsurface drainage, however, is at least partially blocked by a frozen silt cover. That frozen silt is often over saturated. The granular material beneath the silt cover is also frozen but moisture contents are moderate to low.

Thermal conditions

Beneath a seasonally active layer of a foot or two, permafrost is present throughout the study area. The silt cover is universally frozen and segregated ice is sometimes present. The visible ice content is generally limited to 5% or less. However, several one to two foot sections with as much as 25% ice were observed. In the underlying granular material, the soil moisture content is

generally low so frost bonding is sometimes weak and no significant segregated ice was reported.

Cobbles and Boulders

The gravel fraction is generally smaller than one inch in diameter but gravel to two inches was reported in some layers and scattered cobbles are present especially in the gravelly zones.

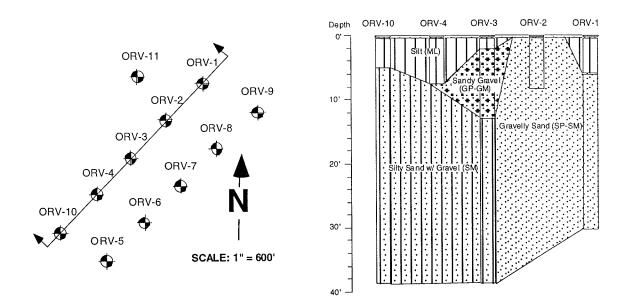
Quality

The area explored is blanketed with a 3 to 4-inch organic mat. Silt overburden depths vary with terrain conditions. At Borings ORV-2 and ORV-9, located on the nose of an east trending ridge, gravelly sand was present directly beneath the organic mat. On each flank of the ridge the silt overburden increased in thickness to about 5 to 7 feet. To the east as much as 14.5 feet of overburden was encountered in Boring ORV-6. The overburden silt sometimes incorporates traces of organic material and scattered pebbles are common.

The lower boundary of the underlying granular material is at least 36 feet deep at Boring ORV-3. The deposit is probably a pro-glacial feature and is composed primarily of a gravelly sand (SP-SM). Grain size distribution within the deposit varies considerably, with interbedded layers of silty sand (SM), sandy gravel (GP-GM) and silty gravel (GM) all present. The silt content of the granular material ranges from 5% to 24% in the 11 holes drilled in 1997. Based on the limited information available, it appears that the silt content increases towards the southern end of the area investigated.

The cross-section on the next page shows the trend in overburden cover and grain size distribution across the deposit from north to south.

Cross-Section of Hotham Peak Investigation



Quantity

The 1997 field work showed that the existing pit can be expanded. The granular material deposit extends at least 1500 feet, north-south and 500 feet, east-west. The existing working face and deep holes at each end of the investigated area indicate that the granular deposit is on the order of 30 feet thick. Additional probe holes should be drilled as the site is being opened, to prove the quantities.

The working face can initially be quite steep because of frost bonding but final slopes should be 1.5 horizontal to 1.0 vertical or flatter. The working area can probably be left open and drained to the east. The natural moisture content of the granular material is generally low, indicating that in-situ densities are relatively high. On this basis, little shrinkage is anticipated. However, if the material is excavated and placed in a frozen condition, significant settlement should be anticipated the first time the material thaws.

The silt cover deposit ranges in thickness from a few inches to as much as 15 feet. Natural moisture contents range from 30% to 65%. If this material is not

utilized as fill, it may be wasted into the worked out portion of the pit or stockpiled on the flanks of the ridge for use in restoration.

Based on aerial photo studies it appears that borrow conditions similar to those reported in the 1973 DOT&PF report extend intermittently for a mile or more to the southwest.

ILLUSTRATIONS

Plate 1

Vicinity Map

APPENDIX: Nazuruk Channel Site

Plate NC-1

Nazuruk Channel Boring Locations

Plates NC-2 through NC-10

Logs of Nazuruk Channel Borings

Plate NC-11

Soil Classification Chart

Plates NC-12 and NC-13

Nazuruk Channel Sample Summary

Plate NC-14

Nazuruk Channel Sieve Summary

Plate NC-15

Nazuruk Channel Compaction Test

APPENDIX: Hotham Peak Site

Plate HP-1

Hotham Peak Boring Locations

Plates HP-2 through HP-15

Logs of Hotham Peak Borings

Plate HP-16

Soil Classification Chart

Plate HP-17

Hotham Peak Sample Summary

Plates HP-18 and HP-19

Hotham Peak Sieve Summary

Plate HP-20

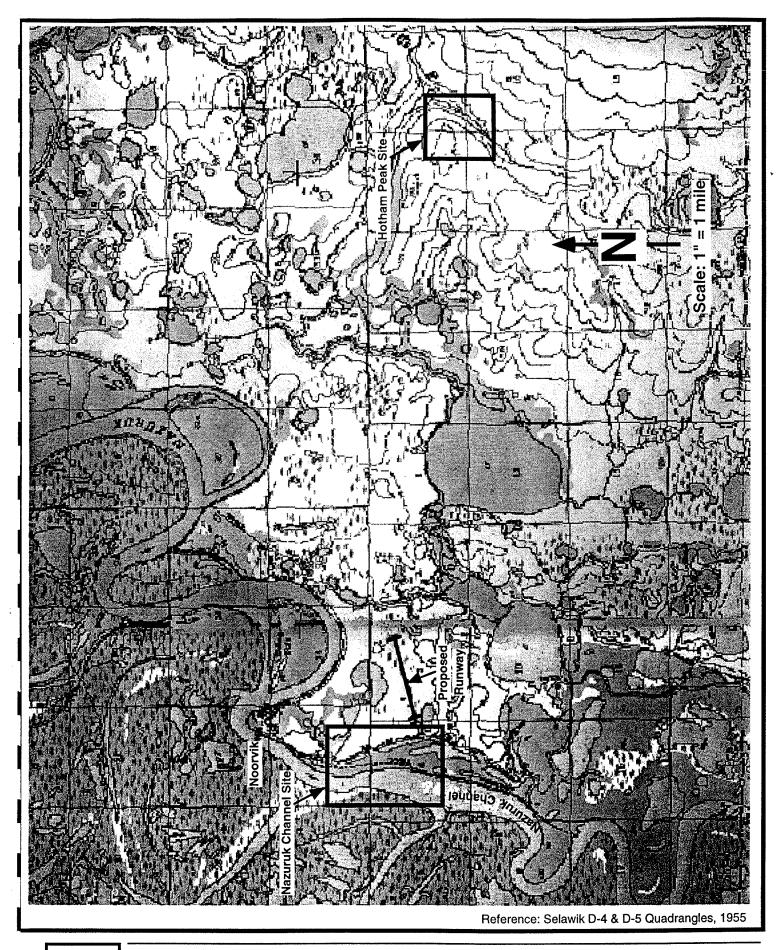
1973 Borings

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1973 laboratory data

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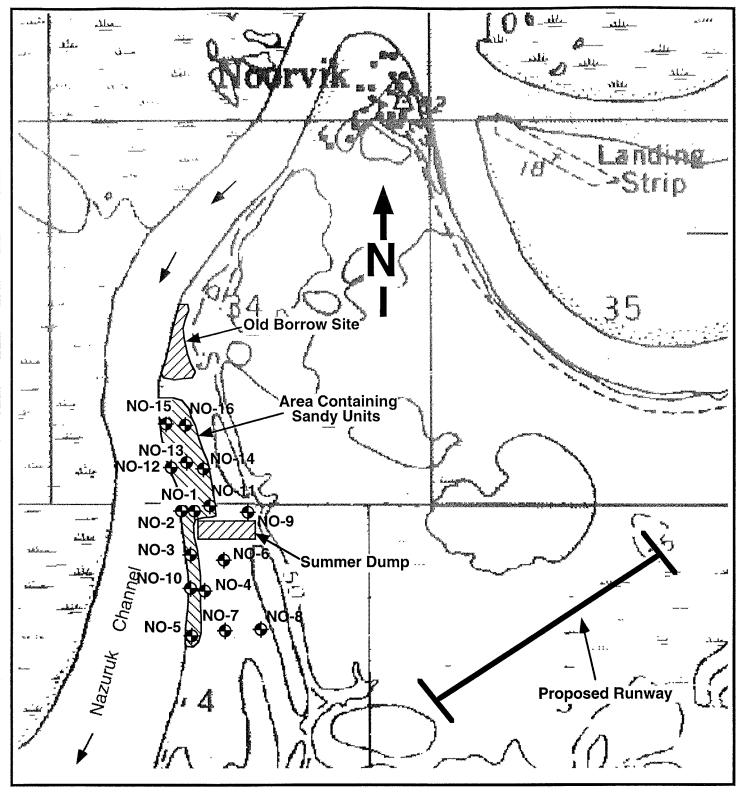
R&M test data from Hotham Peak





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Reference: Enlargement of Selawik (D-5) Quadrangle (1955)

Legend

1996 DMA Borings



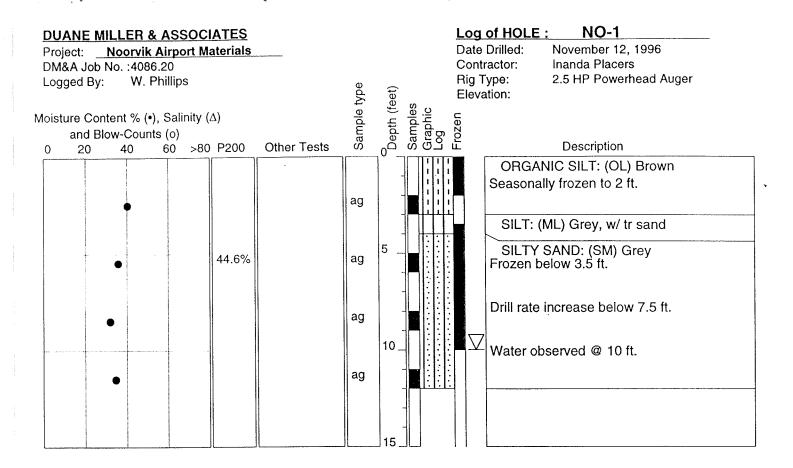
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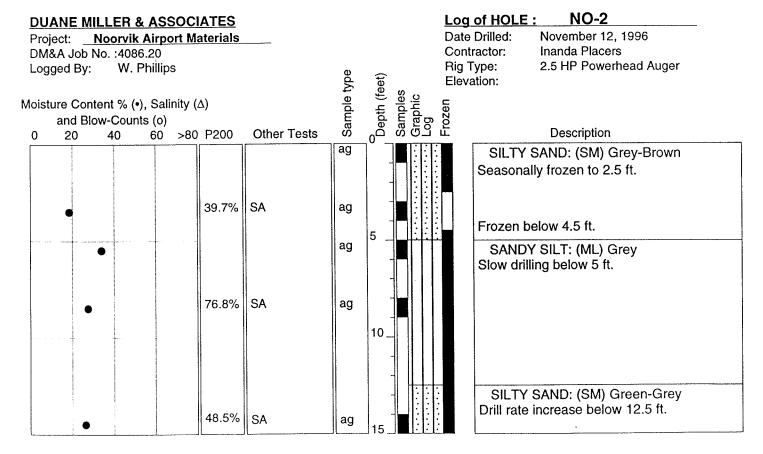


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Job No.: 4086.20 Date : June 1997 BORING LOCATIONS
Nazuruk Channel Material Site
Noorvik, Alaska



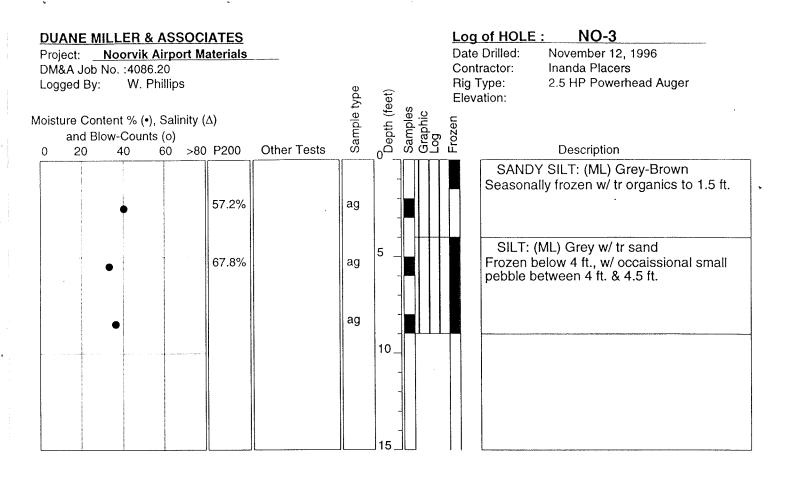


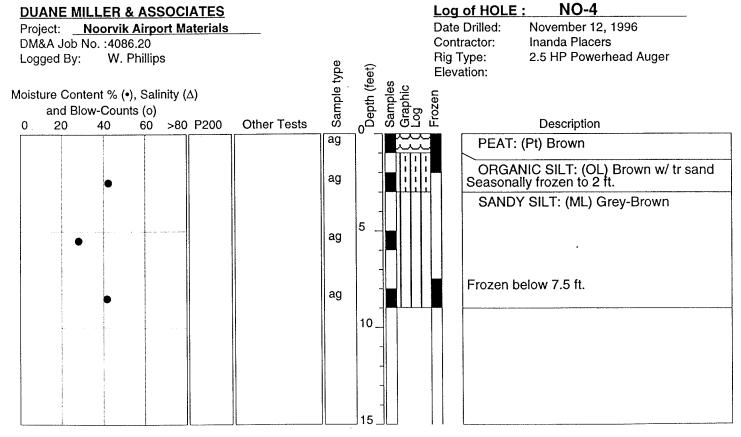


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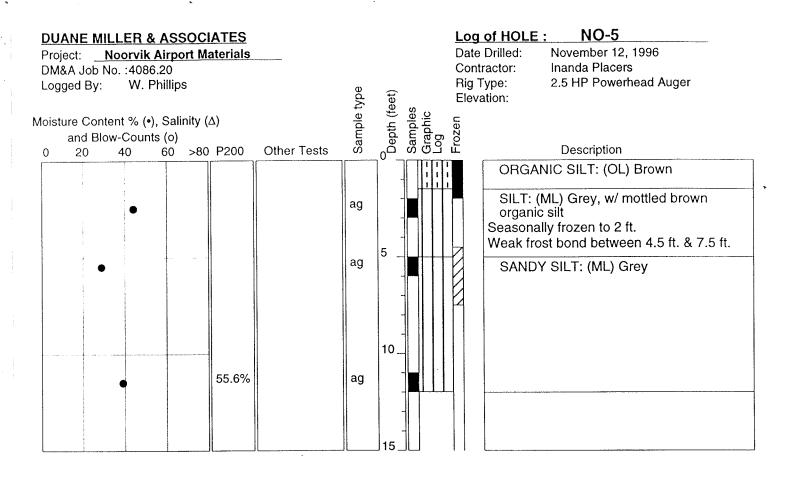


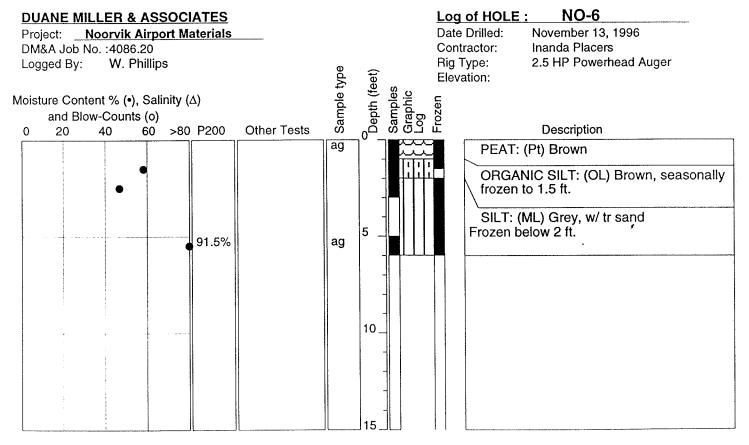


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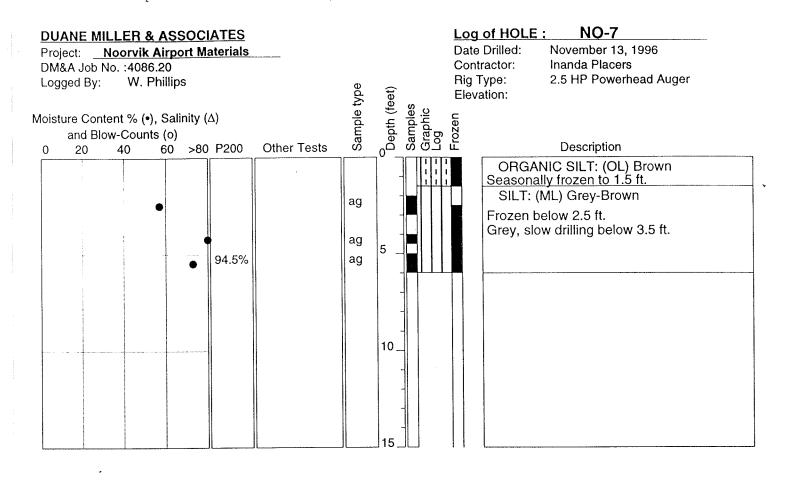


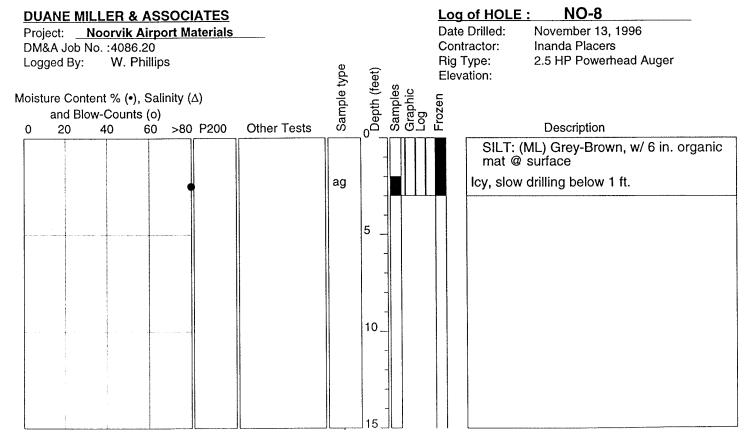


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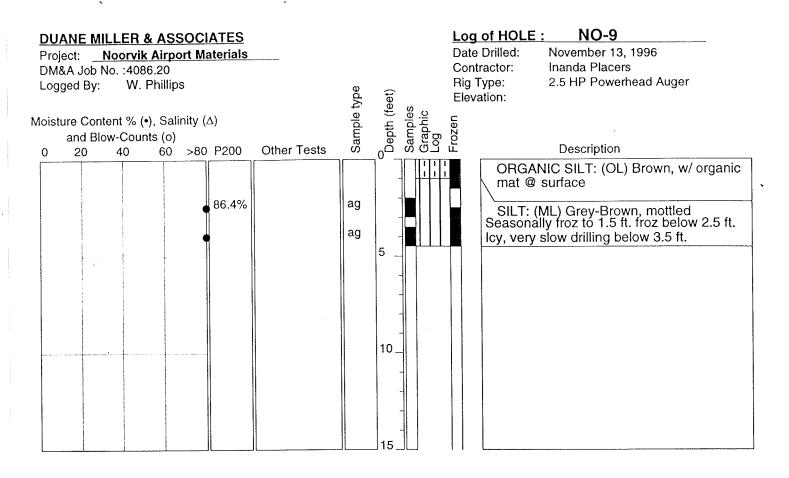


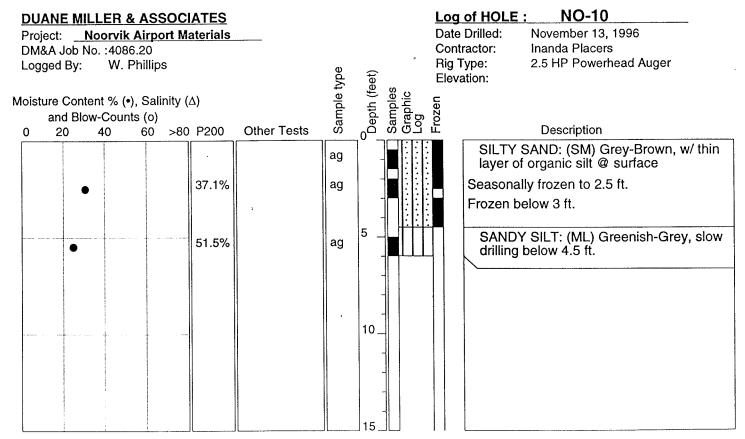


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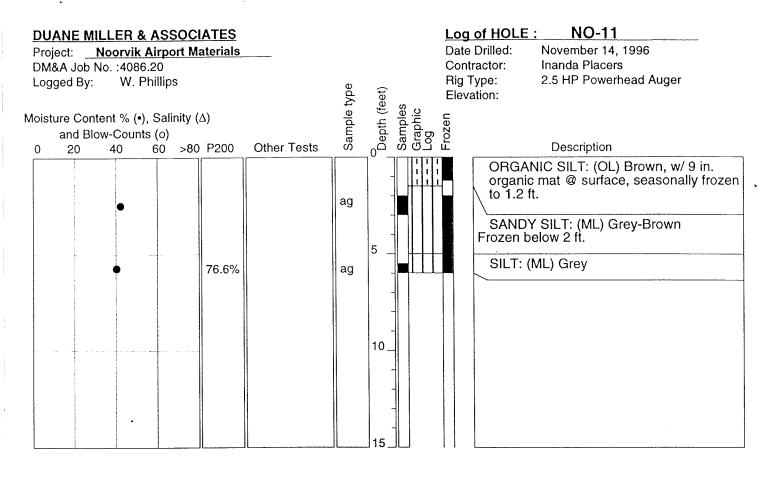


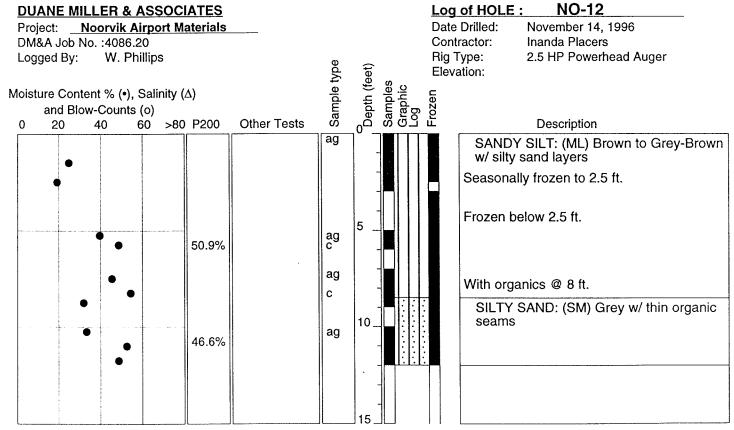


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Job No.: 4086.20 Date: June 1997 LOGS OF BORINGS NO-9 & NO-10 Nazuruk Channel Material Site Noorvik, Alaska



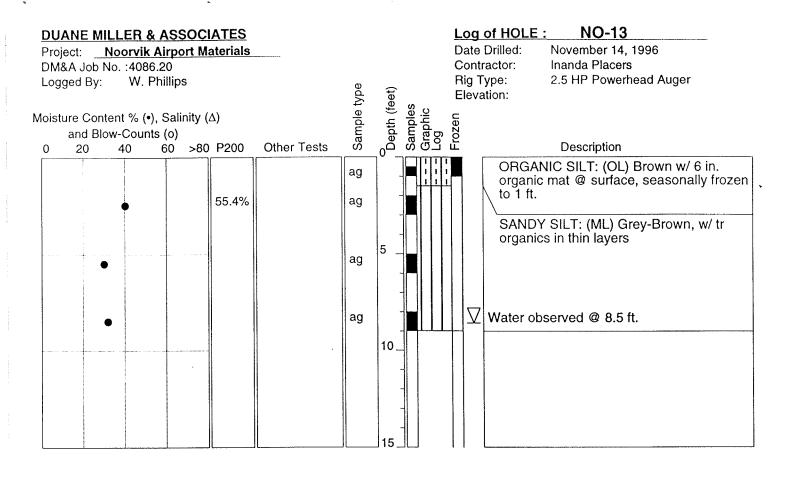


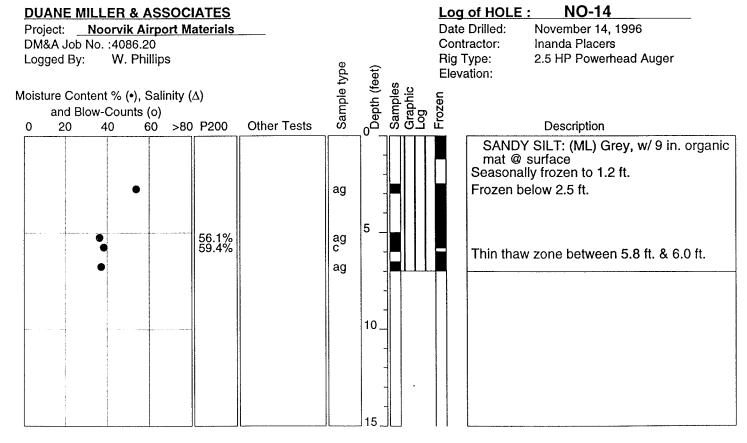


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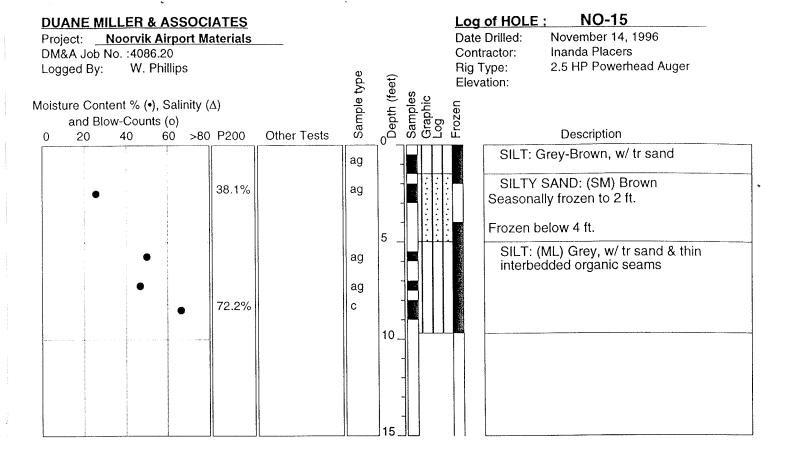


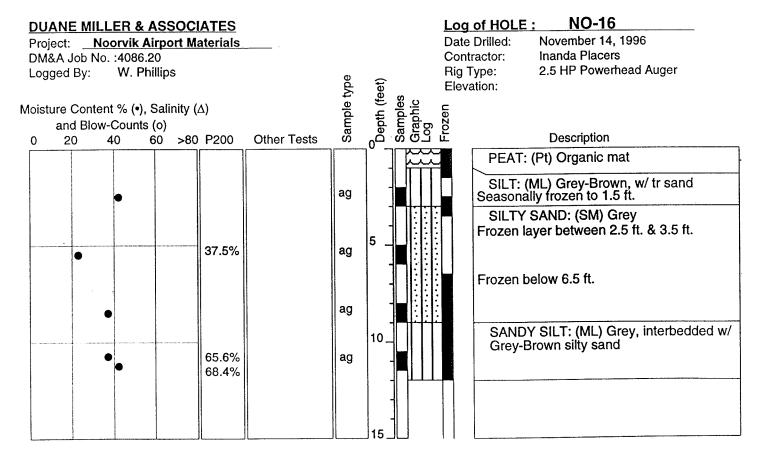


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Nazuruk Channel Material Site
Noorvik, Alaska

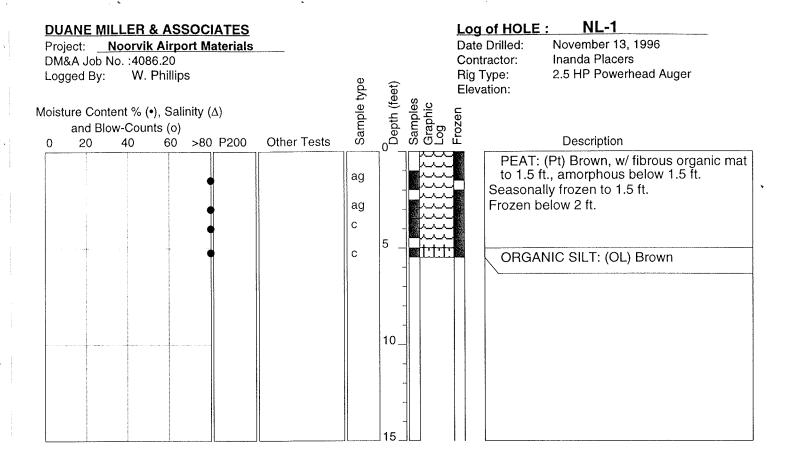






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Job No.: 4086.20 Date: June 1997 LOG OF BORING NL-1 Nazuruk Channel Material Site Noorvik, Alaska

	MAJOR DIVI	SYMBOL		TYPICAL NAMES	
E E		Clean gravels with	GW		Well graded gravels, sandy gravel
.S 9, 0.07	GRAVELS More than half of the coarse fraction is	little or no fines	GP		Poorly graded gravels, sandy gravel
SOILS Sieve,	larger than #4 sieve size, > 4.75 mm.	Gravels with more than 12% fines	GM		Silty gravels, silt sand gravel mixtures
RAINED than #200		than 12% lines	GC		Clayey gravels, clay sand gravel mixtures
SANDS SANDS		Clean sands	sw		Well graded sand, gravelly sand
		with little or no fines	SP		Poorly graded sands, gravelly sand
COAR han 50%	the coarse fraction is smaller than #4 sieve size	Sands with more	SM		Silty sand, silt gravel sand mixtures
More t		than 12% fines	sc		Clayey sand, clay gravel sand mixtures
mm	Plasticity Chart		ML		Inorganic silt and very fine sand, rock flour
OILS 'e, 0.0		SILTS and CLAYS Liquid limit less than 50			Inorganic clay, gravelly and sandy clay, silty clay
IED S	x 40 CH		OL		Organic silts and clay of low plasticity
FINE GRAINED SOILS >50%finer than #200 sleve, 0.07	Plasticity Index O H H H H H H H H H H H H		мн		Inorganic silt
	ML MI	SILTS and CLAYS Liquid limit greater than 50	СН		Inorganic clay, fat clay
F >50%f	Liquid Limit	-5	ОН		Organic silt and clay of high plasticity
HIGHLY ORGANIC SOILS					Peat and other highly organic soil

KEY TO TEST DATA

Dd = Dry Density (pcf) TC = Thaw Consolidation TCf = ThawConsolidation (field) UU = Unconsolidated **Undrained Triaxial** CU = Consolidated **Undrained Triaxial** CD = Consolidated **Drained Triaxial** LL = Liquid Limit PL = Plastic Limit PI = Plastic Index S.G. = Specific Gravity SA = Sieve Analysis MA = Sieve and Hydrometer Analysis OLI = Organic Loss

KEY TO SAMPLE TYPE

Ag = Auger grab Ab = Auger bulk Sh = 2.5" ID split barrel w/340 lb. manual hammer Sha= 2.5" ID split barrel w/340 lb. automatic hammer Tw = Shelby tube

UNIFIED SOIL CLASSIFICATION SYSTEM

GROUP	ICE VISIBILITY	DESC	SYMBOL		
	Cogregated ice not	Poorly bonded or friable			Nf
N ·	Segregated ice not visible by eye	Wall banded	No excess ice	Nb	Nbn
		Well bonded	Excess microscopic ice	IND	Nbe
	Segregated ice is	is Individual ice crystals or inclusions			√x
V	visible by eye and is one inch or less	Ice coatings on p	Vc		
	is one inch or less in thickness	,	Vr		
	Stratified or distinctly oriented ice		Vs		
ICE	Ice greater than one	Ice with soil incl	inclusions ICE + soi		oil type
ICE inch in thickness		Ice without soil in	l	CE	

ICE CLASSIFICATION SYSTEM



Duane Miller & Associates

Arctic & Geotechnical Engineering

Job No.: 4086.20 Date: June 1997 SOIL and ICE CLASSIFICATION and KEY TO DATA

Nazuruk Channel Material Site Noorvik, Alaska Plate

NC-11

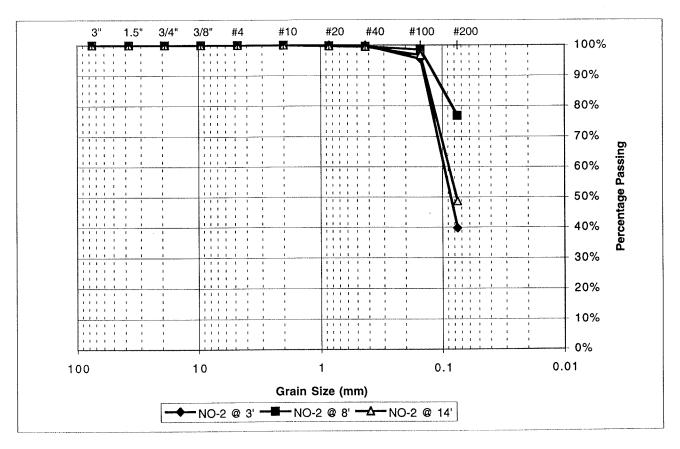
Boring	Sample Depth	Soil Type (USCS)	Frz?	Sample Type	Moisture Content	passing #200	Organic loss	Other Tests
NO-1	2.0 ft.	OL	No	Grab	40.2%			
NO-1	5.0 ft.	SM	Yes	Grab	35.8%	44.6%		
NO-1	8.0 ft.	SM	Yes	Grab	31.8%			
NO-1	11.0 ft.	SM	Yes	Grab	34.4%			
NO-2	0.0 ft.	SM	Yes	Grab				
NO-2	3.0 ft.	SM	No	Grab	18.3%	39.7%		SA
NO-2	5.0 ft.	ML	Yes	Grab	34.2%			
NO-2	8.0 ft.	ML	Yes	Grab	27.2%	76.8%		SA
NO-2	14.0 ft.	SM	Yes	Grab	26.3%	48.5%		SA
NO-3	2.0 ft.	ML	No	Grab	40.5%	57.2%		
NO-3	5.0 ft.	ML	Yes	Grab	33.6%	67.8%		
NO-3	8.0 ft.	ML	Yes	Grab	36.6%			
NO-4	0.0 ft.	Pt	Yes	Grab				
NO-4	2.0 ft.	OL	No	Grab	42.1%			
NO-4	5.0 ft.	ML	No	Grab	28.3%			
NO-4	8.0 ft.	ML	Yes	Grab	41.4%			
NO-5	2.0 ft.	ML	No	Grab	44.1%			
NO-5	5.0 ft.	ML	Yes	Grab	28.5%			
NO-5	11.0 ft.	ML	No	Grab	39.1%	55.6%		
NO-6	0.0 ft.	Pt	Yes	Grab				
NO-6	1.0 ft.	OL	No	Grab	58.1%			
NO-6	2.0 ft.	ML.	Yes	Grab	46.6%			
NO-6	5.0 ft.	ML.	Yes	Grab	117.8%	91.5%		
NO-7	2.0 ft.	ML	No	Grab	57.0%			
NO-7	4.0 ft.	ML.	Yes	Grab	87.0%			
NO-7	5.0 ft.	ML	Yes	Grab	72.8%	94.5%		
NO-8	2.0 ft.	ML	Yes	Grab	92.4%			
NO-9	2.0 ft.	ML	No	Grab	103.0%	86.4%		
NO-9	3.5 ft.	ML	Yes	Grab	110.2%			
NO-10	0.5 ft.	OL	Yes	Grab				
NO-10	2.0 ft.	SM	Yes	Grab	30.6%	37.1%		
NO-10	5.0 ft.	ML	Yes	Grab	24.8%	51.5%		
NO-11	2.0 ft.	ML	Yes	Grab	42.1%		,	
NO-11	5.5 ft.	ML	Yes	Grab	40.0%	76.6%	· · · · · · · · · · · · · · · · · · ·	

SUMMARY OF SAMPLES

Nazuruk Channel Material Site Noorvik, Alaska

Boring	Sample Depth	Soil Type (USCS)	Frz?	Sample Type	Moisture Content	passing #200	Organic loss	Other Tests
NO-12	0.0 ft.	ML	Yes	Grab				
NO-12	1.0 ft.	ML	Yes	Grab	25.2%			
NO-12	2.0 ft.	ML	No	Grab	19.2%			
NO-12	5.0 ft.	ML	Yes	Grab	39.7%			
NO-12	5.5 ft.	ML	Yes	Core	48.7%	50.9%		
NO-12	7.0 ft.	ML.	Yes	Grab	45.6%			
NO-12	8.0 ft.	ML	Yes	Core	54.6%		2.6%	
NO-12	8.5 ft.	SM	Yes	Core	31.7%			
NO-12	10.0 ft.	SM	Yes	Grab	33.1%			
NO-12	10.5 ft.	SM	Yes	Grab	52.4%	46.6%	0.4%	
NO-12	11.5 ft.	SM	Yes	Grab	48.6%			
NO-13	0.5 ft.	OL	Yes	Grab				
NO-13	2.0 ft.	ML	No	Grab	40.2%	55.4%		
NO-13	5.0 ft.	ML	No	Grab	29.8%		0.7%	
NO-13	8.0 ft.	ML	No	Grab	31.7%			
NO-14	2.5 ft.	ML	Yes	Grab	53.9%			:
NO-14	5.0 ft.	ML	Yes	Grab	36.7%	56.1%		
NO-14	5.5 ft.	ML	Yes	Core	38.4%	59.4%		
NO-14	6.5 ft.	ML	Yes	Grab	37.0%			
NO-15	0.5 ft.	ML	Yes	Grab				
NO-15	2.0 ft.	SM	No	Grab	25.7%	38.1%		
NO-15	5.5 ft.	ML	Yes	Grab	49.7%			
NO-15	7.0 ft.	ML	Yes	Grab	46.4%			
NO-15	8.0 ft.	ML	Yes	Core	66.4%	72.2%		
NO-16	2.0 ft.	ML	No	Grab	42.5%			
NO-16	5.0 ft.	SM	Yes	Grab	22.9%	37.5%		
NO-16	8.0 ft.	SM	Yes	Grab	37.4%			
NO-16	10.5 ft.	ML	Yes	Grab	37.2%	65.6%		
NO-16	11.0 ft.	ML	Yes	Grab	42.0%	68.4%		
NL-1	1.0 ft.	Pt	Yes	Grab	217.6%			
NL-1	2.5 ft.	Pt	Yes	Grab	323.5%			
NL-1	3.5 ft.	Pt	Yes	Core	254.2%			
NL-1	5.0 ft.	OL.	Yes	Core	224.9%			

Boring =>	NO-2	NO-2	NO-2
Depth =>	3.0 ft.	8.0 ft.	14.0 ft.
3"	100%	100.0%	100.0%
1 1/2"	100%	100.0%	100.0%
3/4"	100.0%	100.0%	100.0%
3/8"	100.0%	100.0%	100.0%
#4	100.0%	100.0%	100.0%
#10	100.0%	100.0%	100.0%
#20	100.0%	99.8%	99.7%
#40	99.9%	99.6%	99.5%
#100	95.4%	98.5%	96.7%
#200	39.7%	76.8%	48.5%
Analysis of Data			
D10 size =>			
D30 size =>			
D50 size =>	0.085 mm		0.077 mm
D60 size =>	0.097 mm		0.088 mm
Coeff. of Uniformity, Cu =			
Coeff. of Curvature, Cc =			
Gravel (+#4) percentage =	0.0%	0.0%	0.0%
Sand percentage =	60.3%	23.2%	51.4%
Fines percentage =	39.7%	76.8%	48.5%
Unified Soil Class Symbol =	SM	ML	SM



Project = Noorvik Airport

By = M. Hendee

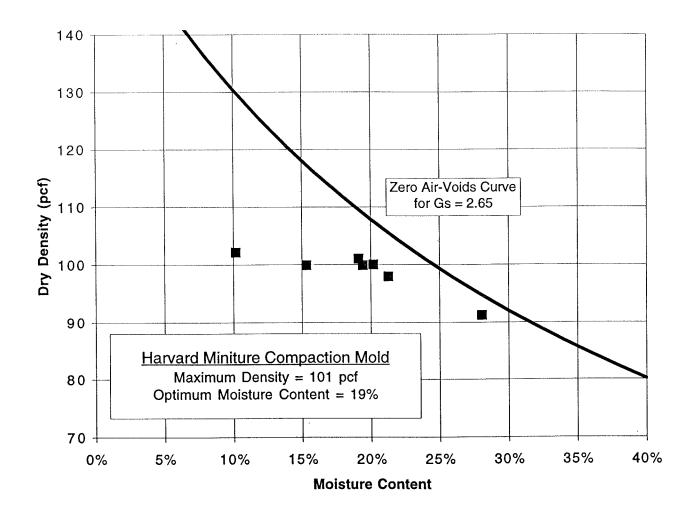
Job No. = 4086.2

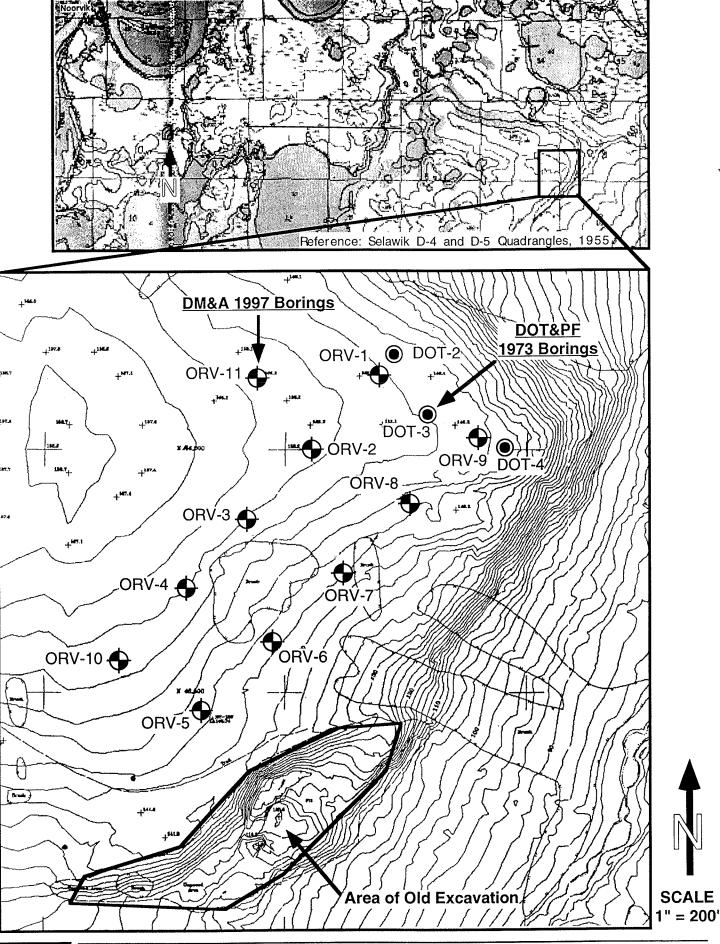
Date = Jan-8-97

Sample Source = Combined samples (NO-1 @ 8.0' & 11.0'),(NO-2 @ 0'),

(NO-12 @ 8.5', 10.0', 11.5'),(NO-16 @ 8.0')

Sample Description = Silty Sand (SM)



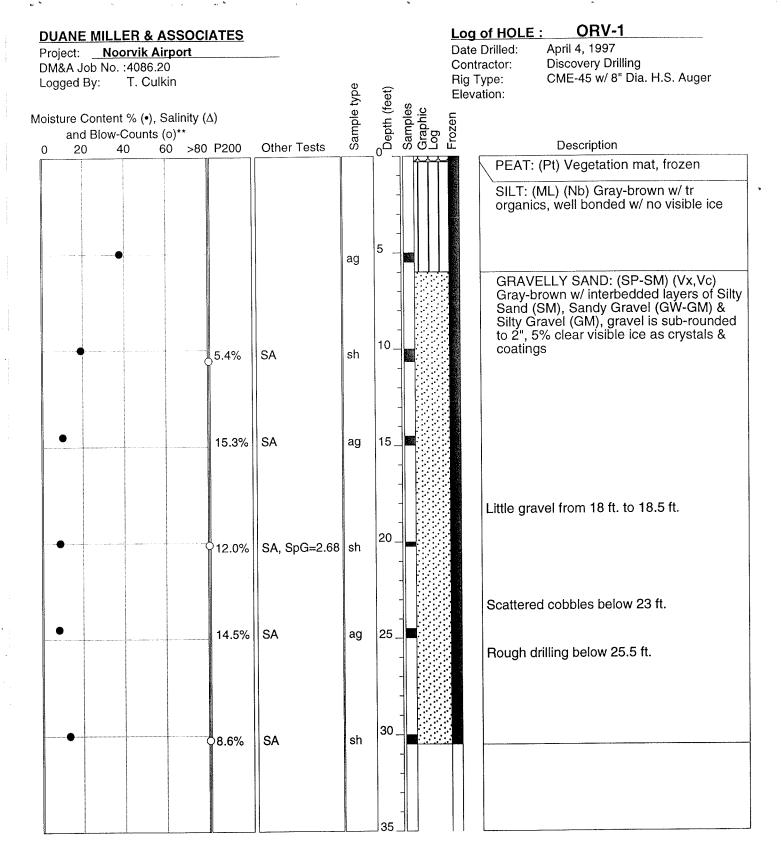




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Job No.: 4086.20 Date : July 1997 BORING LOCATIONS
Hotham Peak Material Site
Noorvik, Alaska

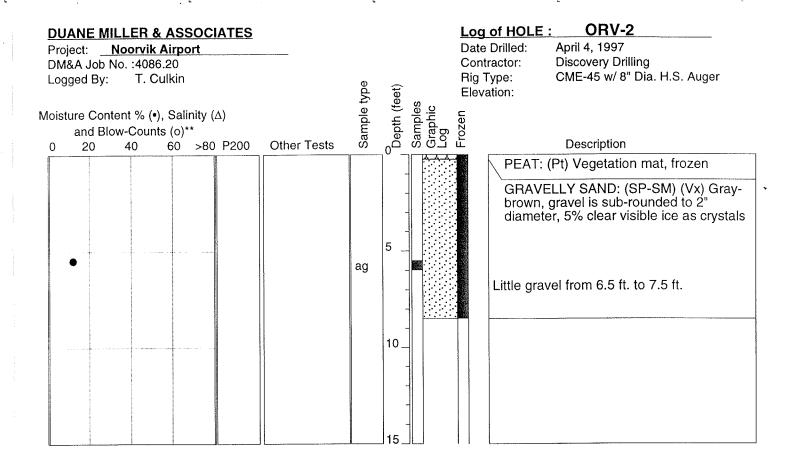
Plate **HP-1**



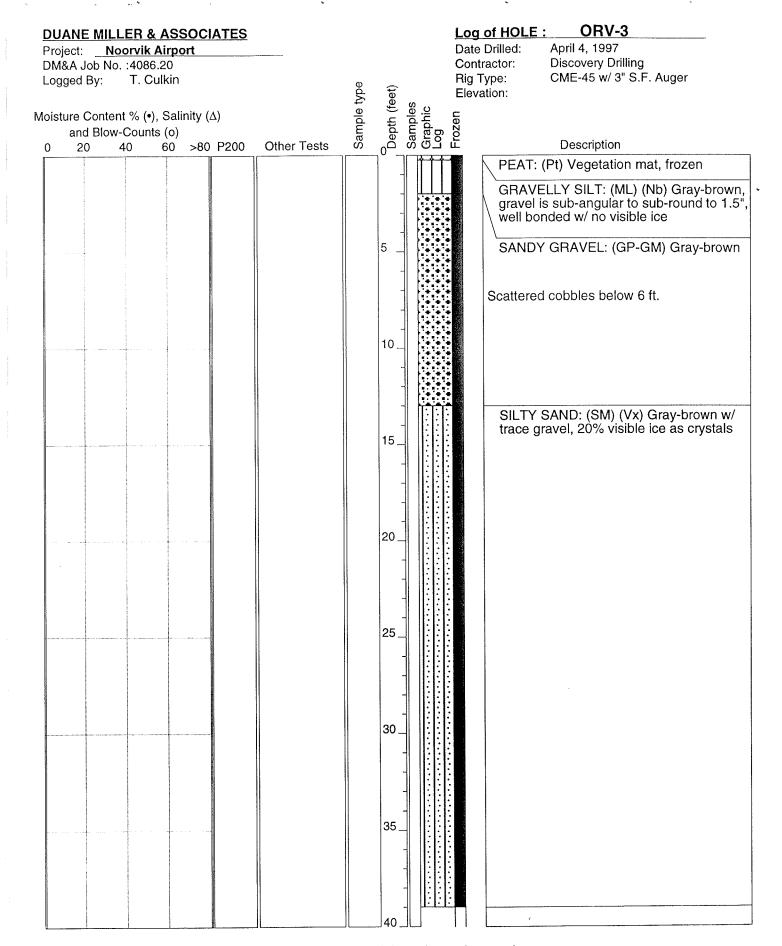
*The field blow counts have been adjusted for hammer weight and sampler annulus



Job No.: 4086.20 Date: June 1997

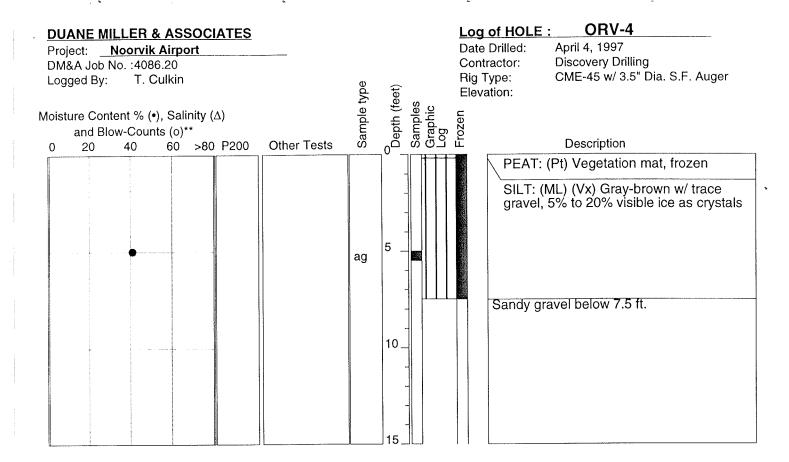






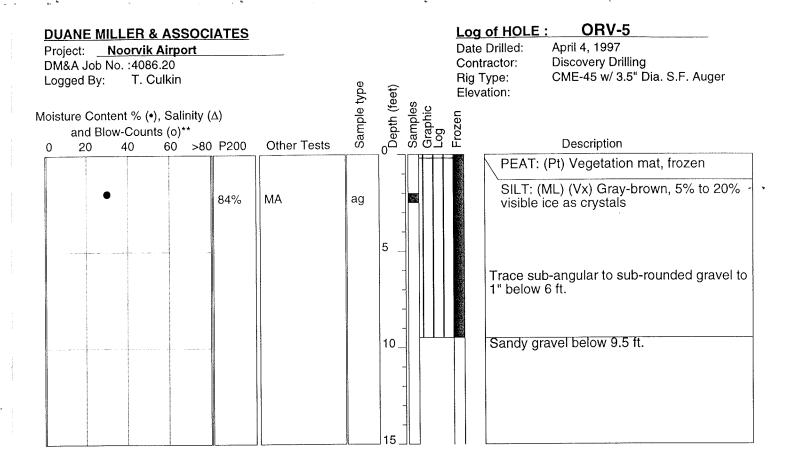


Duane Miller & AssociatesArctic & Geotechnical Engineering

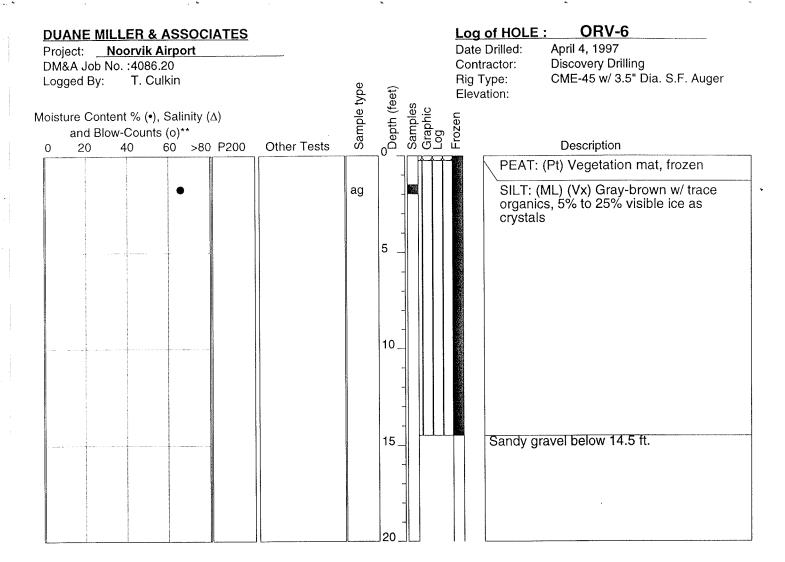




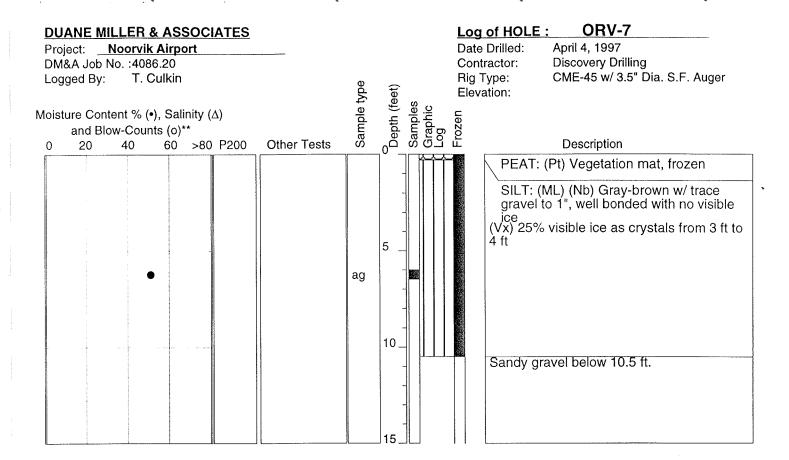
Date: June 1997



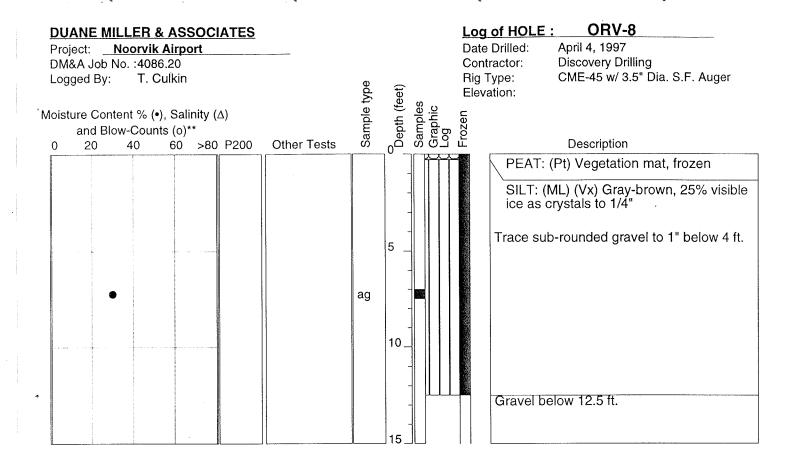






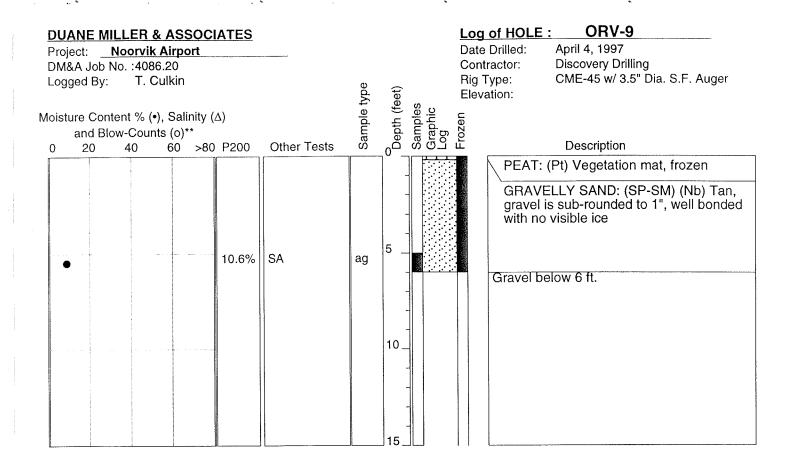








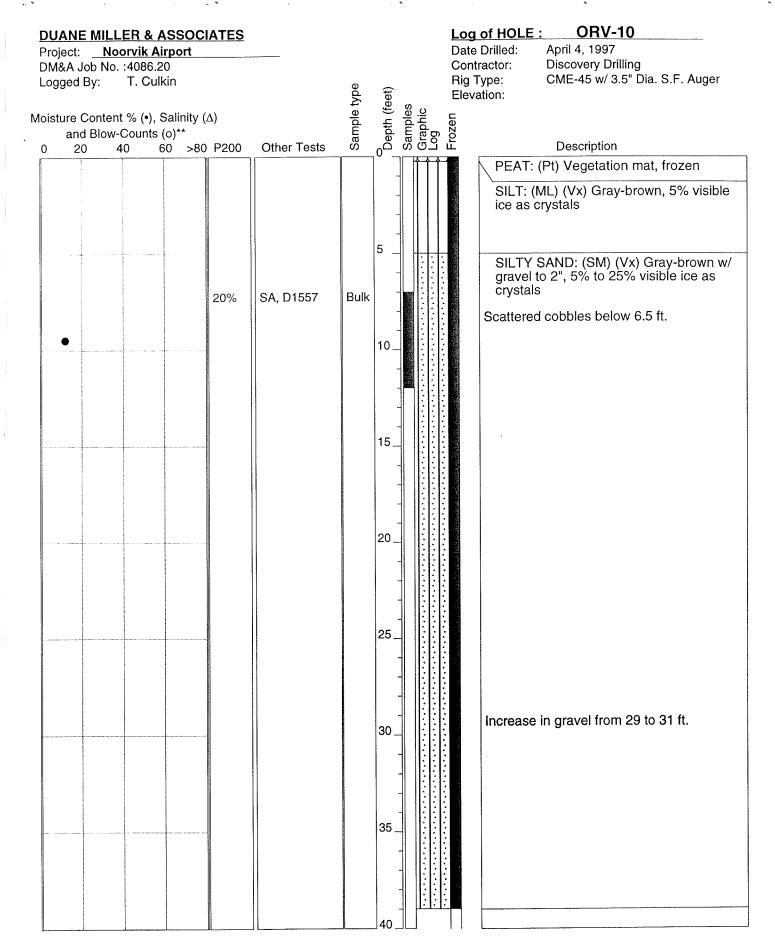
Date: June 1997





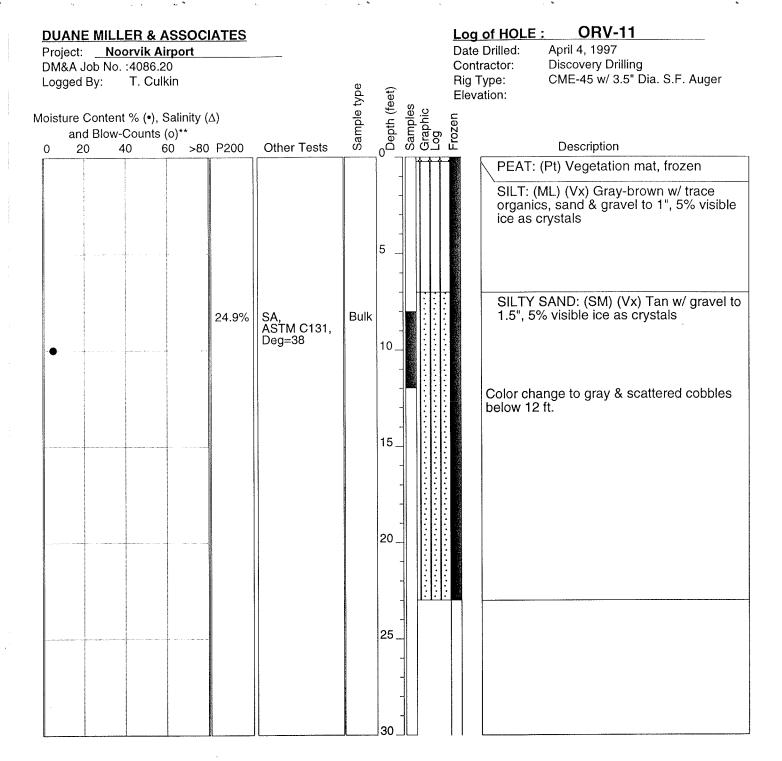
Duane Miller & AssociatesArctic & Geotechnical Engineering
Job No.: 4086.20

Date: June 1997

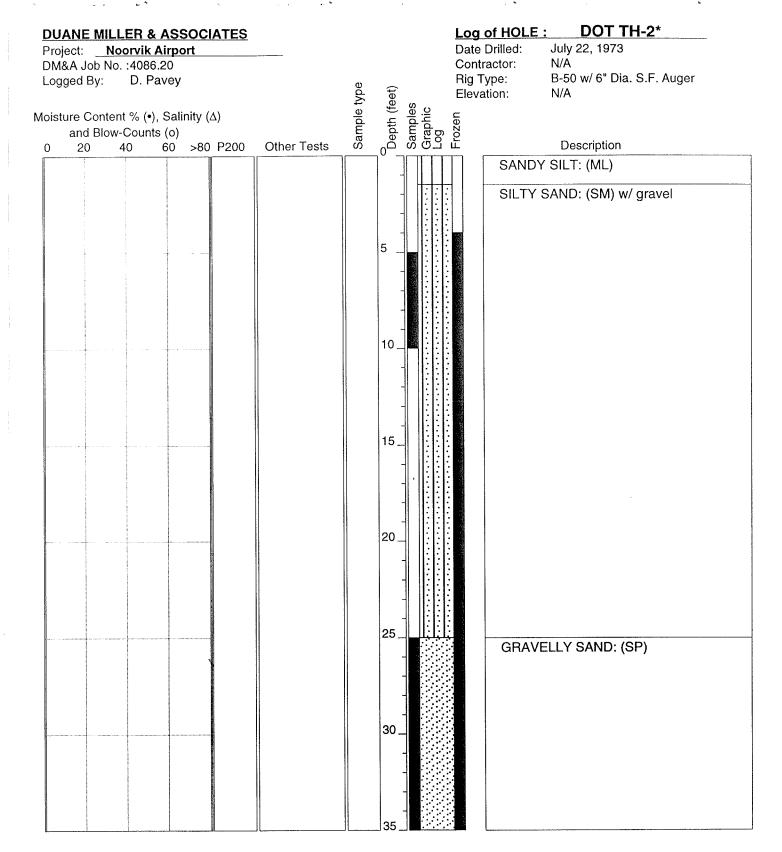




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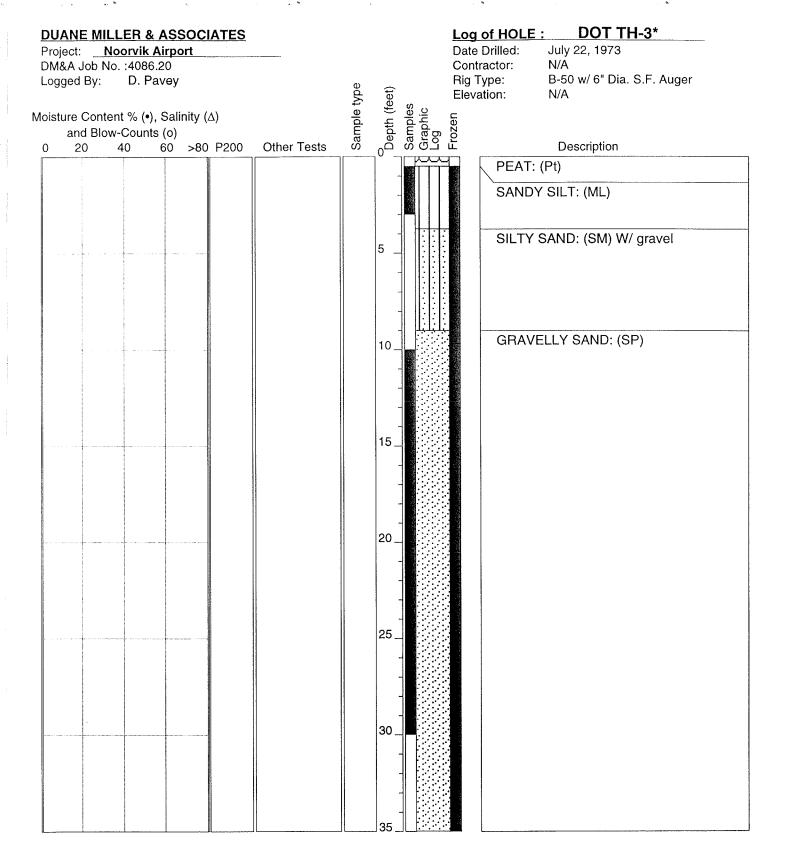






*Re-drafted from original DOT boring log.

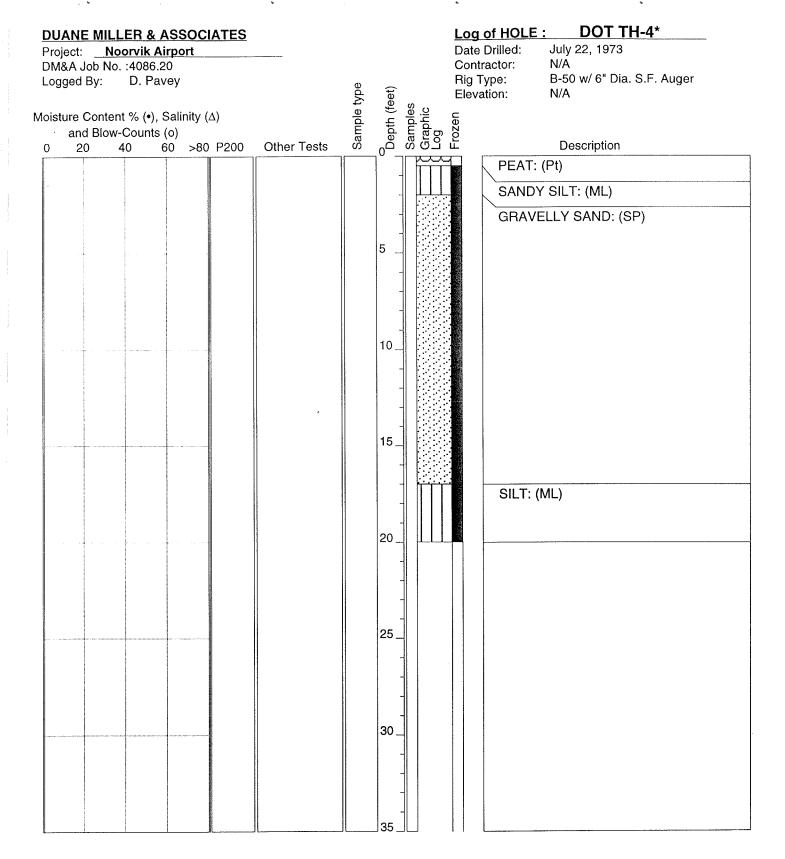




*Re-drafted from original DOT boring log.



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*Re-drafted from original DOT boring log.



	MAJOR DIVI	SIONS	SYM	BOL	TYPICAL NAMES
mm		Clean gravels with	GW		Well graded gravels, sandy gravel
.S 9, 0.07	GRAVELS More than half of the coarse fraction is	little or no fines	GP		Poorly graded gravels, sandy gravel
SOILS © sieve,	larger than #4 sieve size, > 4.75 mm.	Gravels with more than 12% fines	GM		Silty gravels, silt sand gravel mixtures
GRAINED er than #200		man 12 /o mies	GC		Clayey gravels, clay sand gravel mixtures
COARSE GRAINED :		Clean sands with little or no	sw		Well graded sand, gravelly sand
	SANDS More than half of the coarse fraction	fines	SP		Poorly graded sands, gravelly sand
CO than 5(is smaller than #4 sieve size	Sands with more	SM		Silty sand, silt gravel sand mixtures
More t		than 12% fines	sc		Clayey sand, clay gravel sand mixtures
mm	Plasticity Chart		ML		Inorganic silt and very fine sand, rock flour
SOILS eve, 0.07		SILTS and CLAYS Liquid limit less than 50	CL		Inorganic clay, gravelly and sandy clay, silty clay
IED S	x 40 CH		OL	1 1 1	Organic silts and clay of low plasticity
RAIN Ian #20	Plasticity Index		МН		Inorganic silt
FINE GRAINED SOI	ML MI	SILTS and CLAYS Liquid limit greater than 50	СН		Inorganic clay, fat clay
-	Liquid Limit		он	1 1 1 1 1 1 1 1 1 1 1	Organic silt and clay of high plasticity
	HIGHLY ORGANIC	SOILS	Pt		Peat and other highly organic soil

KEY TO TEST DATA

Dd = Dry Density (pcf) TC = Thaw Consolidation TCf = ThawConsolidation (field)
UU = Unconsolidated Undrained Triaxial CU = Consolidated Undrained Triaxial CD = Consolidated **Drained Triaxial** LL = Liquid Limit PL = Plastic Limit PI = Plastic Index S.G. = Specific Gravity SA = Sieve Analysis MA = Sieve and Hydrometer Analysis OLI = Organic Loss

KEY TO SAMPLE TYPE

Ag = Auger grab Ab = Auger bulk Sh = 2.5" ID split barrel w/340 lb. manual hammer Sha= 2.5" ID split barrel w/340 lb. automatic hammer Tw = Shelby tube

UNIFIED SOIL CLASSIFICATION SYSTEM

GROUP	ICE VISIBILITY	DESC	RIPTION	SYMBOL		
	Conversed is a not	Poorly bonded o	r friable	Nf		
N	Segregated ice not visible by eye	Mall banded	No excess ice	Nb	Nbn	
		Well bonded	Excess microscopic ice	IAD	Nbe	
	Segregated ice is	Individual ice cn	Vx			
V	visible by eye and is one inch or less	Ice coatings on	particles	Vc		
	is one inch or less in thickness	Random or irreg	ularly oriented ice	Vr		
	in thickness		nctly oriented ice	Vs		
ICE	Ice greater than one	Ice with soil incl	usions	ICE + soil type		
ICE	inch in thickness	Ice without soil i	nclusions	ICE		

ICE CLASSIFICATION SYSTEM



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Arctic & Geotechnical Engineering

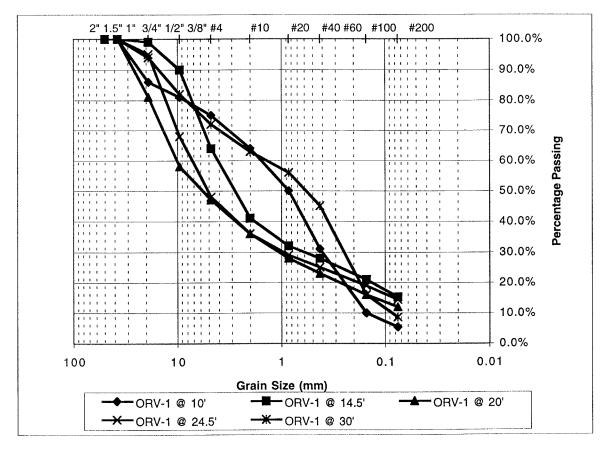
Job No.: 4086.20 Date: June 1997 SOIL and ICE CLASSIFICATION and KEY TO DATA

Hotham Peak Material Site Noorvik, Alaska Plate

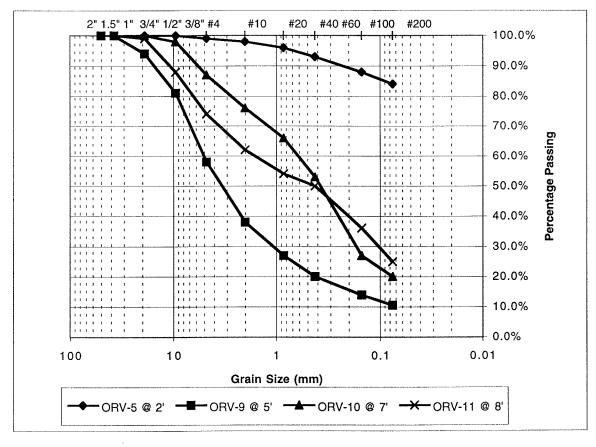
HP-16

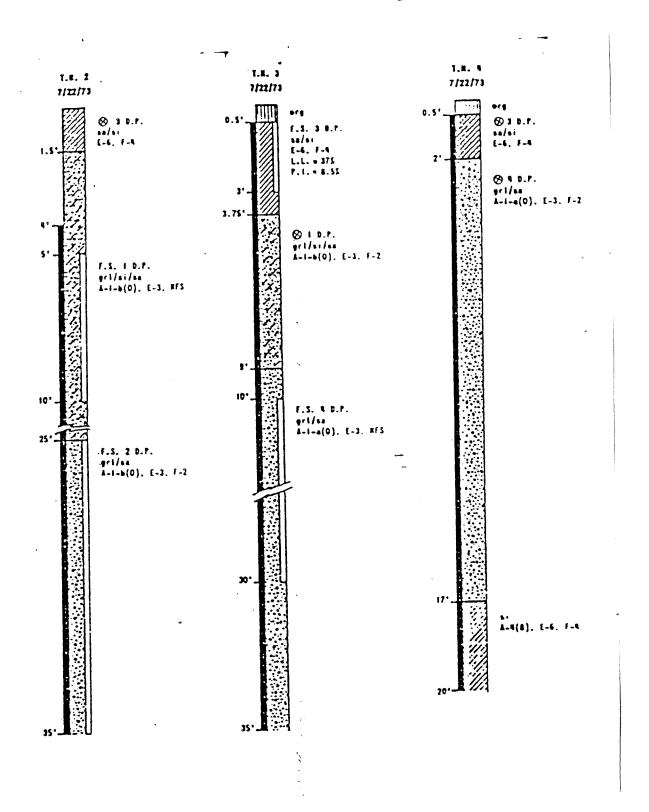
Boring	Sample Depth	Soil Type (USCS)	Frz?	N-value blows/ft	Moisture Content	Passing #200	Other Tests
Dorning	Ворит	(0000)					
ORV-1	5.0 ft.	ML	Yes	Grab	37.6%		
ORV-1	10.0 ft.	SP-SM	Yes	156	19.0%	5.4%	SA
ORV-1	14.5 ft.	SM	Yes	Grab	10.2%	15.3%	SA
ORV-1	20.0 ft.	GW-GM	Yes	156	9.0%	12.0%	SA, SpG=2.68
ORV-1	24.5 ft.	GM	Yes	Grab	8.5%	14.5%	SA
ORV-1	30.0 ft.	SP-SM	Yes	87	13.7%	8.6%	SA
ORV-2	5.5 ft.	SP-SM	Yes	Grab	11.9%		
ORV-4	5.0 ft.	ML	Yes	Grab	41.0%		
ORV-5	2.0 ft.	ML	Yes	Grab	29.8%	84.0%	MA
ORV-6	1.5 ft.	ML	Yes	Grab	65.8%		
ORV-7	6.0 ft.	ML	Yes	Grab	51.2%		
ORV-8	7.0 ft.	ML	Yes	Grab	30.2%		
ORV-9	5.0 ft.	SP-SM	Yes	Grab	8.7%	10.6%	SA
ORV-10	7.0 ft.	SM	Yes	Bulk	12.2%	20.0%	SA, D1557
ORV-11	8.0 ft.	SM	Yes	Bulk	5.3%	24.9%	SA, ASTM C131=28% Loss, Deg=38

Boring =>	ORV-1	ORV-1	ORV-1	ORV-1	ORV-1
Depth =>	10.0 ft.	14.5 ft.	20.0 ft.	24.5 ft.	30.0 ft.
3" =>	100.0%	100.0%	100.0%	100.0%	100.0%
1 1/2" =>	100.0%	100.0%	100.0%	100.0%	100.0%
3/4" =>	86.0%	99.0%	81.0%	95.0%	94.0%
3/8" =>	81.0%	90.0%	58.0%	68.0%	82.0%
#4 =>	75.0%	64.0%	47.0%	48.0%	72.0%
#10 =>	64.0%	41.0%	36.0%	36.0%	63.0%
#20 =>	50.0%	32.0%	28.0%	29.0%	56.0%
#40 =>	31.0%	28.0%	23.0%	25.0%	45.0%
#100 =>	10.0%	21.0%	16.0%	19.0%	16.0%
#200 =>	5.4%	15.3%	12.0%	14.5%	8.6%
Analysis of Data					
D10 size =>	0.150 mm		0.060 mm		0.086 mm
D30 size =>	0.404 mm	0.601 mm	1.053 mm	0.961 mm	0.248 mm
D50 size =>	0.850 mm	2.806 mm	5.738 mm	5.091 mm	0.582 mm
D60 size =>	1.566 mm	4.087 mm	10.090 mm	7.200 mm	1.386 mm
Coeff. of Uniformity, Cu =	10.44		168.17		16.21
Coeff. of Curvature, Cc =	0.70		1.83		0.52
Gravel (+#4) percentage =	25.0%	36.0%	53.0%	52.0%	28.0%
Sand percentage =	69.6%	48.7%	35.0%	33.5%	63.4%
Fines percentage =	5.4%	15.3%	12.0%	14.5%	8.6%
Unified Soil Class Symbol =	SP-SM	SM	GW-GM	GM	SP-SM



Boring =>	ORV-5	ORV-9	ORV-10	ORV-11
Depth =>	2.0 ft.	5.0 ft.	7.0 ft.	8.0 ft.
3" =>	100.0%	100.0%	100.0%	100.0%
1 1/2" =>	100.0%	100.0%	100.0%	100.0%
3/4" =>	100.0%	94.0%	100.0%	99.0%
3/8" =>	100.0%	81.0%	98.0%	88.0%
#4 =>	99.0%	58.0%	87.0%	74.0%
#10 =>	98.0%	38.0%	76.0%	62.0%
#20 =>	96.0%	27.0%	66.0%	54.0%
#40 =>	93.0%	20.0%	53.0%	50.0%
#100 =>	88.0%	14.0%	27.0%	36.0%
#200 =>	84.0%	10.6%	20.0%	24.9%
Analysis of Data				
D10 size =>		0.075 mm		
D30 size =>		1.073 mm	0.169 mm	0.103 mm
D50 size =>		3.361 mm	0.377 mm	0.425 mm
D60 size =>		5.045 mm	0.617 mm	1.615 mm
Coeff. of Uniformity, Cu =		67.27		l
Coeff. of Curvature, Cc =		3.05		
Gravel (+#4) percentage =	1.0%	42.0%	13.0%	26.0%
Sand percentage =	15.0%	47.4%	67.0%	49.1%
Fines percentage =	84.0%	10.6%	20.0%	24.9%
Unified Soil Class Symbol =	ML	SP-SM	SM	SM





Boring Logs for TH-2, TH-3 and TH-4 from Noorvik Materials Investigation by Div. of Aviation, Dec. 4, 1973, which are shown as DOT-2, DOT-3 and DOT-4 on Plate HP-1. See the following page for laboratory data from 1973.

PROJECT HAME	Noory	i k		1973				\$011	LS AI	HALYS	1 S	PR	OJEST	110		3-83-6	5-311-	298-243			
	Depth	Field	Lab	* Nat	·			L AN			PASSI	NG						UNIFIED	OHEZA	Ī	Frost
STATION	in Ft.	No.	No.	Hu · St	3"	2"	1"	3/4"	1/2'	14	•10	*110	1200	• . 02	L.L.	P.L	P.1.		Class	FAA	FSY
T.II. 1A	1.0 - 1.5	1)A76-358	90.2		Organi	c Con	ent #	18.5		100	87	80.0					Org. Sa Si	A-4(8)	Г:-6	F4
1A	2.0 - 2.2	2	1476-359	164.6		Organi	c Con	ent =	40.2									Urg. Si		E-13	
IA.	2.5 - 3.0	3)A76-360	69.6							100	68	\$7.0					Sa Si	A-4(8)	E-6	F4
14	3.5 - 4.0	4)A76-361	57.1							100	93	85.0					Sa Si	A-4(8)	E-6	F4
1.4	5.0 - 5.5	5	1A76-362	38.3							100	99.6	95.7	54.9	31.7	нv	NV	CI SI	A-4(8)	E-6	F4
24	1.5 - 2.0	7	1A76-363	32.3			-				100	99	85.3	29.0	25.9	NV	NV	Sa Si	A-4(8)	E-6	F4
2 A	6.0 - 6.5	9	1A76-364	25.4							100	99.8	71.9		21.8			Sa Si	A-4(7)	E-6	F4
3.4	3.5 - 4.5	11	1A76-365	32.9		Organ	c Con	ent =	6.7		100 .	99.7	94.7	40.2	28.2	NV	NV	Org. Cl Si	A-4(8)	E-6	F4
4.4	2.0 - 3.0	13	1A7G-366	17.0							100	98.1	42.7	6.0		NV	NV	Si Sa	A-4(2)	E-6	F'
4.4	3.0 - 3.5	14	1A76-367	52.7		Organ	c Con	ent =	18.2		100	99.3	95.3			ΝV	ــــــــــــــــــــــــــــــــــــــ	Org. Si	A-4(8)	F:-6	F.
4.4	6.2 - 7.0	15)A76-368	13.5							100	99.8	65.4		25.7	 -		Sa Si	A-4(6)	E-6	F4
5A	1.5 - 2.0	16	1476-369	43.8							100	99.5	78.0	22.0	29.7	NV	NV	Sa Si	A-4(8)	E-6	F4
SA	5.0 - 6.0	17	N76-370	37.9							100	97.6	67.8		31.0	L	L	Sa Si	A-4(7)	E-6	F4 - - - - - - - - - - - - -
6/1	2.0 - 3.0	18	1A 76 - 371	54.1		Organ.	c Con	ent -	10.1		100	99.8	81.0	27.5		NV	NV	Sa Si	X-4(8)		F4
64	1.5 - 5.5	19	IA76-372	39.6							100	99.9	69.8	<u> </u>	27.5	ļ		Sa Si	A-4(7)	E-6	F4
7	3.5 - 4.5	20	1476-373	20.9							<u> </u>	100	\$3.3		27.5		ļ	Sa Si	A-4(4)	1:-0	
Bank Exposure														ļ			<u> </u>		1.7(0)		NI:S
Upstream Island		21	1476-374		<u> </u>	<u> </u>		٠				100	2.5	ļ	NV	NV	КV	Sa	A-3(0)	E-3	
Beach	Surface	22	N76-375			100	90	83	73	46	30	6	1.0	ļ	NV	NV		Sa Grvl	A-1-a(0)	E-2	NFS NFS
Beach "Point"	0.5 - 1.0	23	IA76-376			100	96	95	90	58	44	16	0.3	<u> </u>	NV	NV.		Grv1 Sa	A-1-a(0)	E-2	NES
Beach, N.Selawik Lake	Surface	24	IA76-377			<u> </u>	100	99	95	67	48	21	0.1		NV	NV		Grv1 Sa	A-1-a(0)	E-2	NES
Beach, N.Selawik Lake	1.0 - 2.0	25	A76-378				100	99	97	65	47	23	0.0		NV	NV	NV	Grvl Sa	A-1-a(0)	E-2	NES
Reach, N. Selawik Lake	0 - 1.0	26	1476-379			L	100	98	95	57	37	13	0.1		NV	NV	NV	Grv1 Sa	Λ-1-a(0) Λ-1-a(0)	E-3	NES
Beach, N. Selawik Lake	Surface	27	1476-380				100	99	96	33	14	7	0.3		NV	NV	٧٧	Sa Grvl	V-1-1(0)	+	
																		ļ		 	
																				ļ	
Hotham Peuk Sito TH2	5.0 - 10.0	FS1DP	1473-549				100	99	94	78	61	36	13.7	4.1	NV	NV	NV	Si Grl Sa	A-1-b(0)	E-3	F2
Hothan Penk Site TH3	10.0 - 30.0	FS4DP	1473-552			100	99	97	89	61	10	18	8.6	4.0	NV	NV	NV	Grl Sa w/sSi	A;1:a(0(- E=4	J-F-2
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LABORATORY TEST REPORT

8/2					
R&M CONSULT	ANTS, INC. 9101	VANGUARD DR.	ANCHORAGE, ALASKA 99507	PH 907-522-170	7
CLIENT/PROJECT:	Duane Miller & A	ssociates /	Noorvik Airport (Fld.	Inv. # 2)	R&M PROJECT: 651017
TEST ON:	· · · · · · · · · · · · · · · · · · ·				LAB NO.: 7190
SOURCE:	A CONTRACTOR OF THE PARTY OF TH	***************************************	SUBMITTED BY	: Client	FIELD NO.:
SAMPLED FROM:	The second secon		DATE SAMPLED		DATE REPORTED: 5-16-97
LOCATION:	ORV 5 @ 2	2 to 2.5	DEPTH	•	DATE RECEIVED: 5-15-97

GRAIN S	GRAIN SIZE DISTRIBUTION				FICATION	J	COMPACTION			
SIEVE	% PASS	SPEC.		UNIFIED	AASHTO	FAA				
5"			% + 10				OPTIMUM MOISTURE:			
4"			%+3				MAX. WET DENSITY:			
3"			% GRAVEL				MAX. DRY DENSITY:			
2"		MERCH 11 21	% SAND				CORR. MAX. DRY DENSITY:			
1 1/2 "			% SILT				% FRACTURE:			
1"			% CLAY				METHOD:			
3/4"			FSV				NATURAL DENSITY:			
1/2"			LL				NATURAL MOISTURE:			
3/8"	100		PL				WEIGHT LOOSE:			
#4	99		PI				WEIGHT RODDED:			
#8			CLASS							
#10	98			****						
#16			1		GRA	IN SIZE	DISTRIBUTION CHART			
#20	96		US SIE 4 3	2 1 1/2 1 3/4	1/2 3/8 #4	# 10 #	HYDROMETER 20 # 40 # 60 # 100 # 200 .02	.005 .002		
#30			100			#				
#40	93		<u> </u>							
#50			Z 70 + 60 +							
#60	90		フ 5 50 ₽							
#80			₩ 40 22 30							
#100	88									

10

100

TOTAL WT. TESTED: 161 GMS.

84

60

31

#200

.02MM

.005MM .002MM

COARSE	SPEC	FINE	SPEC	DELETERIOUS MAT.
				MINUS #200 MESH
				SOFT FRAGMENTS
				COAT & LIG. OR L.T.WT.PT.
				CLAY LUMPS
				STICKS & ROOTS
				FRIABLE PARTICLES
				THIN-ELONGATED
				ORGANIC COLOR
				FINENESS MODULUS
				SULFATE SOUNDNESS
				DEGRADATION VALUE
				ABSORPTION
				SPGBULK
1				SPGBULK S.S.D.
				SPGAPPARENT

MOISTURE DENSITY RELATIONSHIP 143 D 142 141 D P 140 E C N F 139 S 138 138 137 136 6 7 4 3 MOISTURE PERCENT TEST POINTS AZERO AIR VIODS 0.00 Zero air voids curve based on an assumed spg. of:

Checked By:

GRAIN SIZE (mm)

0.001

0.01

ORGANIC CONTENT %: L.A. ABRASION LOSS:	Signed By:
REMARKS:	

Tested By: AJS/KCB

LABORATORY TEST REPORT

	M				
R&M CONSULT	ANTS, INC.	9101 VANGUARD DR.	ANCHORAGE, ALASKA 99507	PH 907-522-170	77
CLIENT/PROJECT:	Duane Miller	& Associates /	Noorvik Airport (Fld.	Inv. # 2)	R&M PROJECT: 651017
TEST ON:					LAB NO.: 7190
SOURCE:			SUBMITTED BY	/: Client	FIELD NO.:
SAMPLED FROM:	Boring		DATE SAMPLED	D: N/A	DATE REPORTED: 5-16-97
LOCATION:	ORV 1	0 @ 7.0	DEPTH	+ :	DATE RECEIVED:

GRAIN S	IZE DISTE	RIBUTIO	N	CLASSI	FICATION	V .	COMPACTION			
SIEVE	% PASS	SPEC.		UNIFIED	AASHTO	FAA	<u> </u>			
5"			% + 10				OPTIMUM MOISTURE:	7.5		
4"			%+3				MAX. WET DENSITY:			
3"			% GRAVEL				MAX. DRY DENSITY:	136.5		
2"			% SAND				CORR. MAX. DRY DENSITY:			
1 1/2 "			% SILT				% FRACTURE:			
1"	100		% CLAY				METHOD:			
3/4"	100		FSV				NATURAL DENSITY:			
1/2"	99		LL				NATURAL MOISTURE:			
3/8"	98		PL				WEIGHT LOOSE:			
#4	87		PI				WEIGHT RODDED:			
#8			CLASS							
#10	76									
#16			1		GRA	IN SIZE	DISTRIBUTION CHART			
#20	66		US St 4 3	EVE SIZES 2 1 1/2 1 3/4		# 10 #	#YDROMETER HYDROMETER .02	.005 .002		
#30				 						
#40	53		当 80 +							
	-		-1 2 70 1					3313171 L L		

TOTAL WT. TESTED: 18,865 GMS.

#50 #60

#80

#100 #200

.02MM

.005MM .002MM 36

27

20

COARSE	SPEC	FINE	SPEC	DELETERIOUS MAT.
				MINUS #200 MESH
				SOFT FRAGMENTS
				COAT & LIG. OR L.T.WT.PT.
				CLAY LUMPS
				STICKS & ROOTS
				FRIABLE PARTICLES
				THIN-ELONGATED
				ORGANIC COLOR
				FINENESS MODULUS
				SULFATE SOUNDNESS
				DEGRADATION VALUE
				ABSORPTION
				SPGBULK
				SPGBULK S.S.D.
				SPGAPPARENT

MOISTURE DENSITY RELATIONSHIP
140 D 139 R 138 Y 137 D P 136 E C 135 N F 134 S 133 T 132 Y 131 3 4 5 6 7 8 9 10 TEST POINTS MOISTURE PERCENT A TEST POINTS A ZERO AIR VIODS
Zero air voids curve based on an assumed spg. of: 2.70
Tested By: AJS Checked By: DKJ

the state of the s	Tested By: AJS	Checked By: DKJ
ORGANIC CONTENT %: L.A. ABRASION LOSS: REMARKS:	Signed By:	1. At

LABORATORY TEST REPORT

CLIENT/PROJECT: Duane Miller & Associates / Noorvik Airport (Fid. Inv. #2)	and the last of th		ULTANTS, IN	C. 9101	VANGUARD D	R. ANCHOR	AGE, ALASKA	N 99507 PH 9	907-522-170	07		programs of the State of the St	
TEST ON: Bulk Samples SUBMITTED BY: Client FIELD NO: SAMPLED FROM: N/A DATE SAMPLED: N/A DATE REPORTED: 5 DATE RECEIVED: 5 DATE	CLIENT/F				ssociates	/ Noorvi	k Airport	(Fld. Inv	/. # 2)	R&M	PROJEC	CT: <u>65</u>	1017
SOURCE: Borings SUBMITTED BY: client FIELD NO. SAMPLED FROM: N/A DATE SAMPLED: N/A DATE REPORTED: 5 LOCATION: ORV 10 @ 7* and ORV 11 @ 8* DEPTH: <= DATE REPORTED: 5 Combined CLASSIFICATION SIEVE \$\partial \text{PASS} \text{ SPEC.} UNIFIED ASHTO FAA 5" \$\partial \$\partial \text{ \$\p											LAB N	O.: <u>71</u>	90
SAMPLED FROM: N/A DATE SAMPLED: N/A DATE REPORTED: 5- LOCATION: ORV 10 @ 7' and ORV 11 @ 8' DEPTH: <== DATE RECEIVED: 5						SL	JBMITTE	ED BY:	client				N/A
DEPTH C COMPACTION COMPACTION COMPACTION	SAMPLE		F122 200 200 200 200 200 200 200 200 200			DA	TE SAM	PLED:	N/A	DATE R	EPORTE	ED: 5-1	16-97
STAIN SIZE DISTRIBUTION CLASSIFICATION COMPACTION	1.0	OCATIO	N: ORV		d ORV 11	@ 8'	D	EPTH:	<==	DATE	RECEIVE	ED: 5-	12-97
STRAIN SIZE DISTRIBUTION SUPPLY STRAIN SIZE DISTRIBUTION SUPPLY STRAIN SIZE DISTRIBUTION SUPPLY STRAIN SIZE DISTRIBUTION SUPPLY STRAIN SIZE DISTRIBUTION STRAIN SIZ	L .	00/1110											
SIEVE	OD A INLOI	7F DIOT			CI ASSI	EICATIO	Ni	1	(OMPAC	CTION		
5" %+10 OPTIMUM MOISTURE: 4" %+3 MAX. WET DENSITY: 2" % SAND OORAMA. DRY DENSITY: 11/2" % SILT % FRACTURE: 11" % CLAY METHOD: 11/2" LL NATURAL DENSITY: 1/2" LL WEIGHT LOOSE: 1/4" PI WEIGHT CODE: 1/4" PI WEIGHT CODE: 1/4" PI WEIGHT CODE: 1/4" PI WEIGHT CODE: 1/4" WEIGHT LOOSE: 1/4" PI WEIGHT LOOSE: 1/4" WEIGHT LOOSE: 1/4" PI WEIGHT LOO				1			·	-	•				
4"		% PASS	SPEC.		UNIFIED	AASHTO	FAA	ODTIMU	LSION M	TIDE:			
3"													
2"							ļ					*******	
1 1/2 "													
1" % CLAY METHOD:					1					I DENSIII.			M 8-44/17/07
3/4"													
1/2"	-									(37) /-			
3/8"				· · · · · · · · · · · · · · · · · · ·									
##4							-						
#10					<u> </u>								
#10								WEIGH	RODDE	<u>-υ:</u>			
#16				CLASS	<u> </u>								
#10				_		GP/	NN SIZE	DISTRIR	LITION	CHART			
#20				live en	EVE SIZES	OIV.	AIIN SIZL	DIGITAL	OHOR		7		
#40	#20			4 3	2 1 1/2 1 3/4	1/2 3/8 #4	# 10 #	20 #40 #60	#100 # 200	02	.005	.002	
#200 .02MM .005MM .002MM TOTAL WT. TESTED: COARSE SPEC FINE SPEC DELETERIOUS MAT. MINUS #200 MESH SOFT FRAGMENTS COAT & LIG. OR L.T.WT.PT. CLAY LUMPS STICKS & ROOTS FRIABLE PARTICLES FRIABLE PARTICLES THIN-ELONGATED ORGANIC COLOR FINENESS MODULUS SULFATE SOUNDNESS T SABORPTION 0 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0	#30			1 111									
#200 .02MM .005MM .002MM TOTAL WT. TESTED: GMS. COARSE SPEC FINE SPEC DELETERIOUS MAT. MINUS #200 MESH SOFT FRAGMENTS COAT & LIG. OR L.T.WT.PT. CLAY LUMPS STICKS & ROOTS FRIABLE PARTICLES FRIABLE PARTICLES THIN-ELONGATED ORGANIC COLOR FINENESS MODULUS SULFATE SOUNDNESS T CAST SULFATE SOUNDNESS SULFATE SOUNDNESS T CAST SULFATE SUL	#40												
#200 .02MM	#50												
#200 .02MM .005MM .002MM TOTAL WT. TESTED: GMS. COARSE SPEC FINE SPEC DELETERIOUS MAT. MINUS #200 MESH SOFT FRAGMENTS COAT & LIG. OR L.T.WT.PT. CLAY LUMPS STICKS & ROOTS FRIABLE PARTICLES FRIABLE PARTICLES THIN-ELONGATED ORGANIC COLOR FINENESS MODULUS SULFATE SOUNDNESS T CAST SULFATE SOUNDNESS SULFATE SOUNDNESS T CAST SULFATE SUL	#60												
#200 .02MM .005MM .002MM TOTAL WT. TESTED: GMS. COARSE SPEC FINE SPEC DELETERIOUS MAT. MINUS #200 MESH SOFT FRAGMENTS COAT & LIG. OR L.T.WT.PT. CLAY LUMPS STICKS & ROOTS FRIABLE PARTICLES FRIABLE PARTICLES THIN-ELONGATED ORGANIC COLOR FINENESS MODULUS SULFATE SOUNDNESS T CAST SULFATE SOUNDNESS SULFATE SOUNDNESS T CAST SULFATE SUL	#80]									
100 10 1 0.1 0.01	#100												
.02MM	#200			1 .				шш			0.01	1_1	{ 0,001
.002MM	.02MM			100		10					0.01		0,001
COARSE SPEC FINE SPEC DELETERIOUS MAT.	.005MM							GRAIN SIZ	.c. (IIIII)				
COARSE SPEC FINE SPEC DELETERIOUS MAT.	.002MM												
MINUS #200 MESH SOFT FRAGMENTS COAT & LIG. OR L.T.WT.PT. CLAY LUMPS STICKS & ROOTS FRIABLE PARTICLES THIN-ELONGATED ORGANIC COLOR FINENESS MODULUS SULFATE SOUNDNESS THIN-ELONDRESS THIN-E	TOTAL V	VT. TEST	ED:	G	MS.	_					T. 1		
SOFT FRAGMENTS COAT & LIG. OR L.T.WT.PT. CLAY LUMPS STICKS & ROOTS FRIABLE PARTICLES THIN-ELONGATED ORGANIC COLOR FINENESS MODULUS SULFATE SOUNDNESS DEGRADATION VALUE ABSORPTION R	COARSE	SPEC	FINE	SPEC DI	LETERIOU	1							++
COAT & LIG. OR L.T.WT.PT. CLAY LUMPS STICKS & ROOTS FRIABLE PARTICLES THIN-ELONGATED ORGANIC COLOR FINENESS MODULUS SULFATE SOUNDNESS DEGRADATION VALUE ABSORPTION TO COAT & LIG. OR L.T.WT.PT. Y D D D D D D D D D D D D				Mil	NUS #200 MESH								
CLAY LUMPS STICKS & ROOTS FRIABLE PARTICLES THIN-ELONGATED ORGANIC COLOR FINENESS MODULUS SULFATE SOUNDNESS TT ABSORPTION CLAY LUMPS DEGRADATION VALUE ABSORPTION D				sc	FT FRAGMENT	s F	₹						-
STICKS & ROOTS FRIABLE PARTICLES THIN-ELONGATED ORGANIC COLOR FINENESS MODULUS SULFATE SOUNDNESS DEGRADATION VALUE ABSORPTION STICKS & ROOTS E N S S S S S S S S S S S S S S S S S				cc	AT & LIG. OR L	.T.WT.PT. 🛚 🕽						1	
FRIABLE PARTICLES THIN-ELONGATED ORGANIC COLOR FINENESS MODULUS SULFATE SOUNDNESS DEGRADATION VALUE ABSORPTION FRIABLE PARTICLES N N N N N N N N N N N N N N N N N N N				CL	AY LUMPS	Ì					1-1-		4-4-
THIN-ELONGATED N				ST	ICKS & ROOTS	ן נ							1
ORGANIC COLOR FINENESS MODULUS SULFATE SOUNDNESS T Y ABSORPTION ORGANIC COLOR S I I I I I I I I I I I I I I I I I I				FR	HABLE PARTICL								+
FINENESS MODULUS SULFATE SOUNDNESS DEGRADATION VALUE ABSORPTION FINENESS MODULUS T T Y ABSORPTION				TH	IIN-ELONGATED) N	4					4-4-	
SULFATE SOUNDNESS 38 DEGRADATION VALUE ABSORPTION T Y D D D D D D D D D D D D D D D D D				OF	RGANIC COLOR	5	3						1
38 DEGRADATION VALUE ABSORPTION				FI	NENESS MODU	LUS	I <u> </u>						
ABSORPTION				sı	JLFATE SOUND	NESS 7	Γ					44	4-4
ABSORPTION D D D D D D D D D D D D D D D D D D D	38			DE	GRADATION V	ALUE \	Y					1-1-	
SPGBULK P				AE	SORPTION								
				SF	PGBULK		P					_	1-1
SPGBULK S.S.D. C		 		SF	PGBULK S.S.D	. (\perp
SPGAPPARENT F MOISTURE - PERCENT				SF	PGAPPARENT		F	MC	DISTUF				
Tested By: KCB Checked By: DKJ						-	ested By:	КСВ		Checked	By: D	KJ	
ORGANIC CONTENT %:	ORGANIC	CONTEN	Т%:					11	ſ	1 1			
L.A. ABRASION LOSS: 28 (grading D) Signed By:				(grading D)		S	signed By:		(·SI	toe		
REMARKS:												-	